

## Influence of sine material gradients on delamination in multilayered beams

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**Abstract.** The present paper deals with delamination fracture analyses of the multilayered functionally graded non-linear elastic Symmetric Split Beam (SSB) configurations. The material is functionally graded in both width and height directions in each layer. It is assumed that the material properties are distributed non-symmetrically with respect to the centroidal axes of the beam cross-section. Sine laws are used to describe the continuous variation of the material properties in the cross-sections of the layers. The delamination fracture is analyzed in terms of the strain energy release rate by considering the balance of the energy. A comparison with the J-integral is performed for verification. The solution derived is used for parametric analyses of the delamination fracture behavior of the multilayered functionally graded SSB in order to evaluate the effects of the sine gradients of the three material properties in the width and height directions of the layers and the location of the crack along the beam width on the strain energy release rate. The solution obtained is valid for two-dimensional functionally graded non-linear elastic SSB configurations which are made of an arbitrary number of lengthwise vertical layers. A delamination crack is located arbitrary between layers. Thus, the two crack arms have different widths. Besides, the layers have individual widths and material properties.

**Keywords:** multilayered beam; delamination fracture; material non-linearity; two-dimensional sine material gradient

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### 1. Introduction

Functionally graded materials are a promising alternative to homogeneous structural materials mainly because by gradual varying the material properties along one or more spatial directions during manufacturing, one can get optimum performance of functionally graded structural members and components to external loads (Bensaid and Kerboua 2017, Bensaid *et al.* 2017, Bohidar *et al.* 2014, Gasik 2010, Hirai and Chen 1999, Koizumi 1993, Markworth *et al.* 1995, Mortensen and Suresh 1995, Nemat-Allal *et al.* 2011, Neubrand and Rödel 1997, Uslu Uysal and Kremzer 2015, Uslu Uysal 2016, Uslu Uysal and Güven 2016, Uslu Uysal 2017). In structural applications of functionally graded materials, fracture is a critical failure mode. The inhomogeneous character of functionally graded materials imposes a significant difficulty in

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fracture analyses of functionally graded structures.

Multilayered materials are manufactured by bonding of layers of different materials. Recently, multilayered materials have been used as advanced structural materials in various engineering applications (Markov and Dinev 2005). One of the major drawbacks of the layered materials is their high susceptibility to delamination fracture (Dolgov 2005, 2016). Delamination, i.e., separation of layers, considerably reduces the strength and stiffness of multilayered structural members and components and even may lead to catastrophic failure (Guadette *et al.* 2001, Narin 2006, Hsueh *et al.* 2009, Szekrenyes 2010, Szekrenyes 2016a, b).

An interesting work on delamination fracture behavior of multilayered beam configurations has been published by Narin (2006). The analysis has been carried-out by assuming linear-elastic mechanical behavior of the material in each layer. Thus, the principles of linear-elastic fracture mechanics have been applied. The delamination has been studied in terms of the strain energy release rate. Methods for analyzing the strain energy release rate have been developed with considering the influence of residual stresses. The effect of temperature differences on the delamination fracture behavior has also been analyzed and discussed. Delamination behavior of several multilayered beam configurations has been investigated.

Delamination fracture in multilayered linear-elastic beam configurations subjected to four-point bending has been analyzed by Hsueh *et al.* (2009). A solution to the strain energy release rate has been obtained by applying methods of linear-elastic fracture mechanics. The solution can be used for multilayered four-point bending beam systems with any number of layers. It is assumed that the delamination cracking can occur at any interface.

Delamination fracture in multilayered functionally graded beam structures has been analyzed recently also with considering the non-linear behaviour of the material by Rizov (2017a), (2017b), (2017c), Rizov (2018). The material non-linearity has been described by a power-law stress-strain relation. By considering the complementary strain energy, analytical solutions for the strain energy release rate have been derived assuming that only one material property (the coefficient of the power law stress-strain relation) is functionally graded.

The purpose of the present paper is to develop a delamination fracture analysis of the multilayered functionally graded non-linear elastic SSB configuration assuming that the material is functionally graded in both width and height directions of each layer. The three material properties, which are involved in the non-linear stress-strain relation, vary continuously in the cross-section of each layer according to a sine law (the properties are distributed non-symmetrically with respect to the centroidal axes of the beam cross-section). The solution for the strain energy release rate, derived by analyzing the balance of the energy, is applied to evaluate the influences of sine material gradients along the width and height of the layers, the delamination crack location along the beam width and the non-linear mechanical behavior of the functionally graded material on the delamination fracture.

## 2. Analysis of the strain energy release rate

The multilayered functionally graded SSB configuration that is analyzed in the present paper is shown schematically in Fig. 1. The beam is made of lengthwise vertical layers. Perfect adhesion is assumed between the layers. The number of the layers is arbitrary. Also, the layers have different widths and material properties. Besides, the functionally graded material in each layer exhibits non-linear mechanical behaviour. The beam cross-section is a rectangle of width,  $b$ , and height,  $h$ .

The beam length is  $2l$ . A notch of depth,  $b_2$ , is introduced in the right-hand lateral surface of the beam in order to generate conditions for delamination fracture. A delamination crack is located symmetrically with respect to the beam mid-span. The crack length is  $2a$ . The delamination crack is located arbitrary along the beam width. The widths of the left-hand and the right-hand crack arms are denoted by  $b_1$  and  $b_2$ , respectively. The boundaries of the left-hand crack arm are  $l - a \leq x_3 \leq l + a$ ,  $-b/2 \leq y_3 \leq b_1 - b/2$  and  $-h/2 \leq z_3 \leq h/2$ . The right-hand crack arm has the following boundaries:  $l - a \leq x_3 \leq l + a$ ,  $b_1 - b/2 \leq y_3 \leq b/2$  and  $-h/2 \leq z_3 \leq h/2$ .

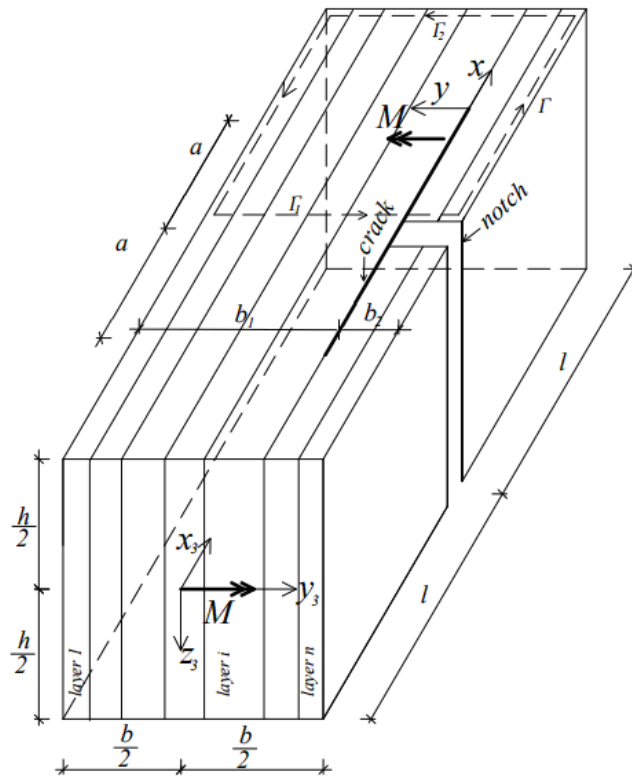


Fig. 1 The geometry of the multilayered functionally graded SSB configuration

The notch divides the right-hand crack arm in two symmetric segments of length,  $a$ , each. The beam is loaded by two moments,  $M$ , applied at the end sections of the beam (Fig. 1). Obviously, the two segments of the right-hand crack arm are free of stresses. It should be noted that the normal stresses induced by the bending of the SSB configuration around the horizontal centroidal axis of the beam cross-section generate a combination between mode two and mode three cracking.

Due to the symmetry, only half of the beam,  $l \leq x_3 \leq 2l$ , is analyzed (Fig. 1).

The delamination fracture is studied in terms of the strain energy release rate,  $G$ , by analyzing

the balance of the energy. By assuming an increase of the crack length,  $\delta a$ , the balance of the energy is written as

$$M\delta\varphi = \frac{\partial U}{\partial a} \delta a + Gh\delta a, \quad (1)$$

where  $\delta\varphi$  is the increase of the rotation of the end section of the beam,  $U$  is the strain energy cumulated in half of the beam. Form (1), one arrives at

$$G = \frac{M}{h} \frac{\partial\varphi}{\partial a} - \frac{1}{h} \frac{\partial U}{\partial a}. \quad (2)$$

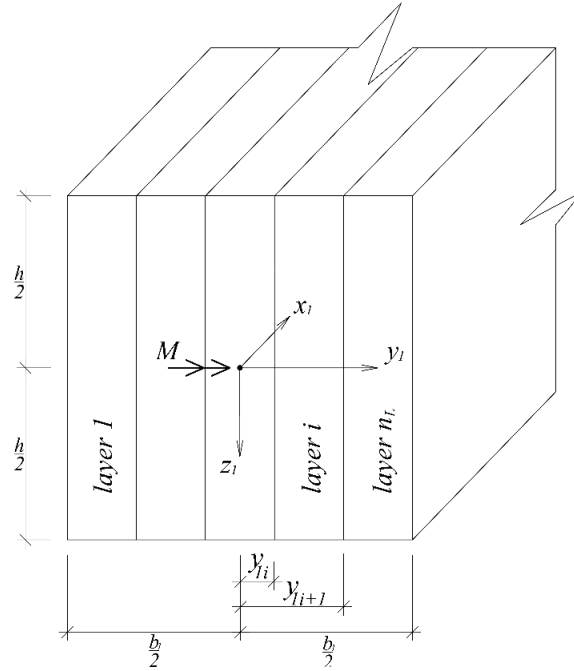


Fig. 2 The cross-section of the left-hand crack arm in the beam mid-span

It should be mentioned that the present analysis is performed assuming validity of the small strains hypothesis. Besides, the present analysis holds for non-linear elastic behaviour of the material. However, the analysis is applicable also for elastic-plastic behaviour if the beam under consideration undergoes active deformation, i.e., if the external loading increases only (Chakrabarty 2006, Lubliner 2006).

The rotation of the end section of the beam,  $\varphi$ , is determined by the Castigliano's theorem for structures exhibiting material non-linearity

$$\varphi = \frac{\partial U^*}{\partial M}, \quad (3)$$

where  $U^*$  is the complementary strain energy cumulated in half of the beam.

Since the two segments of the right-hand crack arm are free of stresses (Fig. 1), the complementary strain energy cumulated in half of the beam is written as

$$U^* = U_L^* + U_U^*, \quad (4)$$

where  $U_L^*$  and  $U_U^*$  are the complementary strain energies cumulated, respectively, in the left-hand crack arm and the un-cracked beam portion,  $l + a \leq x_3 \leq 2l$ .

By addition of the complementary strain energies cumulated in the layers of the left-hand crack arm,  $U_L^*$  is expressed as

$$U_L^* = a \sum_{i=1}^{i=n_L} \int_{y_{li}}^{y_{li+1}} \int_{-\frac{h}{2}}^{\frac{h}{2}} u_{0L_i}^* dy_1 dz_1, \quad (5)$$

where  $n_L$  is the number of the layers in the left-hand crack arm,  $y_{li}$  and  $y_{li+1}$  are the coordinates, respectively, of the left-hand and the right-hand lateral surfaces of the  $i$ -th layer (Fig. 2),  $u_{0L_i}^*$  is the complementary strain energy density in the same layer,  $y_1$  and  $z_1$  are the centroidal axes of the cross-section of the left-hand crack arm.

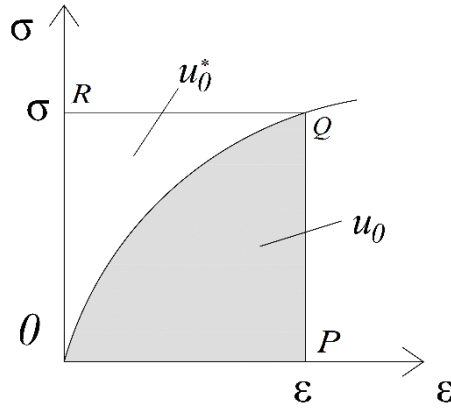


Fig. 3 Schematic of a non-linear stress-strain curve (the strain energy and the complementary strain energy densities are denoted by  $u_0$  and  $u_0^*$ , respectively)

In principle, the complementary strain energy density is equal to the area,  $OQR$ , which supplements the area,  $OPQ$ , enclosed by the stress-strain curve to a rectangle (Fig. 3). Thus,  $u_{0L_i}^*$  is written as

$$u_{0L_i}^* = \sigma_i \varepsilon - u_{0L_i}, \quad (6)$$

where  $\sigma_i$  is the distribution of the longitudinal normal stresses in the  $i$ -th layer of the left-hand crack arm,  $\varepsilon$  is the distribution of the longitudinal strains,  $u_{0L_i}$  is the strain energy density in the same layer. The strain energy density is equal to the area,  $OPQ$ , enclosed by the stress-strain curve (Fig. 3). Therefore,  $u_{0L_i}$  is expressed as

$$u_{0L_i} = \int_0^{\varepsilon} \sigma_i d\varepsilon. \quad (7)$$

In the present paper, the non-linear mechanical behaviour of the functionally graded material in the  $i$ -th layer is described by the following stress-strain relation (Lukash 1998)

$$\sigma_i = E_i \left[ 1 - \left( 1 - \frac{\varepsilon}{\beta_i} \right)^{r_i} \right], \quad (8)$$

where  $E_i$ ,  $\beta_i$  and  $r_i$  are material properties.

By combining of (7) and (8), one obtains

$$u_{0L_i} = E_i \varepsilon + \frac{E_i \beta_i}{r_i + 1} \left( 1 - \frac{\varepsilon}{\beta_i} \right)^{r_i + 1} - \frac{E_i \beta_i}{r_i + 1}. \quad (9)$$

From (6), (8) and (9), one derives

$$u_{0L_i}^* = -E_i \varepsilon \left( 1 - \frac{\varepsilon}{\beta_i} \right)^{r_i} - \frac{E_i \beta_i}{r_i + 1} \left( 1 - \frac{\varepsilon}{\beta_i} \right)^{r_i + 1} + \frac{E_i \beta_i}{r_i + 1}. \quad (10)$$

It assumed that the material properties,  $E_i$ ,  $\beta_i$  and  $r_i$ , which are involved in the stress-strain relation (8), are functionally graded in both width and height directions in each layer. The continuous variation of  $E_i$ ,  $\beta_i$  and  $r_i$  in the cross-section of the  $i$ -th layer is described by the following sine laws

$$E_i = E_{0i} \left[ 1 + f_{E_i} \sin \left( \frac{y_1 - y_{1i}}{y_{1i+1} - y_{1i}} \frac{\pi}{2} \right) + g_{E_i} \sin \left( \frac{z_1 + \frac{h}{2} \frac{\pi}{2}}{h} \right) \right], \quad (11)$$

$$\beta_i = \beta_{0i} \left[ 1 + f_{\beta_i} \sin \left( \frac{y_1 - y_{1i}}{y_{1i+1} - y_{1i}} \frac{\pi}{2} \right) + g_{\beta_i} \sin \left( \frac{z_1 + \frac{h}{2} \frac{\pi}{2}}{h} \right) \right], \quad (12)$$

$$r_i = r_{0i} \left[ 1 + f_{r_i} \sin \left( \frac{y_1 - y_{1i}}{y_{1i+1} - y_{1i}} \frac{\pi}{2} \right) + g_{r_i} \sin \left( \frac{z_1 + \frac{h}{2} \frac{\pi}{2}}{h} \right) \right], \quad (13)$$

where  $f_{E_i}$ ,  $f_{\beta_i}$  and  $f_{r_i}$  are material properties which govern the material gradients, respectively,

of  $E_i$ ,  $\beta_i$  and  $r_i$  along  $y_1$  axis. The material gradients of  $E_i$ ,  $\beta_i$  and  $r_i$  along  $z_1$  axis are governed by  $g_{E_i}$ ,  $g_{\beta_i}$  and  $g_{r_i}$ , respectively. Formulae (11), (12) and (13) indicate that  $E_i$ ,  $\beta_i$  and  $r_i$  are distributed non-symmetrically with respect to  $y_1$  and  $z_1$ . It should be noted that the sine laws provide smooth material gradients in both width and height directions in each layer.

The distribution of the longitudinal strains is analyzed assuming validity of the Bernoulli's hypothesis for plane sections, since the span to height ratio of the beam under consideration is large. It should also be noted that since the beam is loaded in pure bending, the only non-zero strain is  $\varepsilon$ . Thus, according to the small strains compatibility equations,  $\varepsilon$  is distributed linearly in the cross-section. Hence, the strain distribution in the cross-section of the left-hand crack arm is written as

$$\varepsilon = \varepsilon_{C_1} + \kappa_{y_1} y_1 + \kappa_{z_1} z_1, \quad (14)$$

where  $\varepsilon_{C_1}$  is the strain in the centre of the cross-section,  $\kappa_{y_1}$  and  $\kappa_{z_1}$  are the curvatures of left-hand crack arm in the  $x_1y_1$  and  $x_1z_1$  planes, respectively.

The following equations for equilibrium of the cross-section of the left-hand crack arm are used to determine  $\varepsilon_{C_1}$ ,  $\kappa_{y_1}$  and  $\kappa_{z_1}$

$$N_1 = \sum_{i=1}^{i=n_L} \int_{y_{1i}}^{y_{1i+1}} \int_{-\frac{h}{2}}^{\frac{h}{2}} \sigma_i dy_1 dz_1, \quad (15)$$

$$M_{y_1} = \sum_{i=1}^{i=n_L} \int_{y_{1i}}^{y_{1i+1}} \int_{-\frac{h}{2}}^{\frac{h}{2}} \sigma_i z_1 dy_1 dz_1, \quad (16)$$

$$M_{z_1} = \sum_{i=1}^{i=n_L} \int_{y_{1i}}^{y_{1i+1}} \int_{-\frac{h}{2}}^{\frac{h}{2}} \sigma_i y_1 dy_1 dz_1, \quad (17)$$

where  $N_1$  is the axial force,  $M_{y_1}$  and  $M_{z_1}$  are the bending moments about  $y_1$  and  $z_1$  axes, respectively. It is obvious that (Fig. 2)

$$N_1 = 0, \quad M_{y_1} = M, \quad M_{z_1} = 0. \quad (18)$$

By substituting of (8), (11), (12), (13) and (14) in (15), (16) and (17), one derives

$$N_1 = h \sum_{i=1}^{i=n_L} E_{0i} \left( \chi_i c_i + \frac{1}{2} q_i \nu_i + \frac{1}{3} \nu_i \phi_i g_i \right), \quad (19)$$

$$M_{y_1} = \frac{h^3}{12} \sum_{i=1}^{i=n_L} E_{0_i} \left[ (\mu_i \lambda_i + \zeta_i \eta_i) \chi_i + \frac{1}{2} (v_i \lambda_i + \zeta_i \phi_i) \nu_i \right], \quad (20)$$

$$M_{z_1} = h \sum_{i=1}^{i=n_L} E_{0_i} \left[ \frac{1}{2} \nu_i c_i + \frac{1}{3} q_i \mathcal{G}_i + \frac{1}{4} v_i \phi_i (y_{1i+1}^4 - y_{1i}^4) \right], \quad (21)$$

where

$$\psi_i = \frac{\pi}{2(y_{1i+1} - y_{1i})}, \quad (22)$$

$$\gamma_i = -y_{1i} \pi, \quad (23)$$

$$\delta = \frac{\pi}{2h}, \quad (24)$$

$$\theta_i = \beta_{0_i} (1 + f_{\beta_i} \sin \gamma_i + g_{\beta_i} 0.707), \quad (25)$$

$$\omega_i = 1 - \frac{\varepsilon_{C_1}}{\theta_i}, \quad (26)$$

$$\varphi_i = r_{0_i} (1 + f_{r_i} \sin \gamma_i + g_{r_i} 0.707), \quad (27)$$

$$\phi_i = \omega_i^{\varphi_i} \left( \frac{\varphi_i}{\omega_i} \frac{\kappa_{y_1} \theta_i - \varepsilon_{C_1} \beta_{0_i} f_{\beta_i} \psi_i \cos \gamma_i}{\theta_i^2} - r_{0_i} f_{r_i} \psi_i \cos \gamma_i \lg \omega_i \right), \quad (28)$$

$$\lambda_i = \omega_i^{\varphi_i} \left( \frac{\varphi_i}{\omega_i} \frac{\kappa_{z_1} \theta_i - \varepsilon_{C_1} \beta_{0_i} g_{\beta_i} \delta 0.707}{\theta_i^2} - r_{0_i} g_{r_i} \delta 0.707 \lg \omega_i \right), \quad (29)$$

$$\chi_i = y_{1i+1} - y_{1i}, \quad (30)$$

$$\nu_i = y_{1i+1}^2 - y_{1i}^2, \quad (31)$$

$$\mathcal{G}_i = y_{1i+1}^3 - y_{1i}^3, \quad (32)$$

$$\mu_i = 1 + f_{E_i} \sin \gamma_i + 0.707 g_{E_i}, \quad (33)$$

$$v_i = f_{E_i} \psi_i \cos \gamma_i, \quad (34)$$



$$\zeta_i = 0.707 g_{E_i} \delta, \quad (35)$$

$$c_i = \mu_i \eta_i + \zeta_i \lambda_i \frac{h^2}{12}, \quad (36)$$

$$q_i = \mu_i \phi_i + \nu_i \eta_i. \quad (37)$$

It should be noted that at  $n_L = 1$ ,  $f_{E_i} = 0$ ,  $g_{E_i} = 0$ ,  $f_{\beta_i} = 0$ ,  $g_{\beta_i} = 0$ ,  $f_{r_i} = 0$  and  $g_{r_i} = 1$ , formulae (19), (20) and (21) transform in

$$N_1 = E_{0_i} \varepsilon_{C_1} b h, \quad (38)$$

$$M_{y_1} = E_{0_i} \frac{b h^3}{12} \kappa_{z_1}, \quad (39)$$

$$M_{z_1} = E_{0_i} \frac{b^3 h}{12} \kappa_{y_1}. \quad (40)$$

The fact that (38), (39) and (40) are exact matches of the equations for equilibrium of linear-elastic homogeneous beam of rectangular cross-section of width,  $b_1$ , and height,  $h$ , indicates the consistency of Eqs. (19), (20) and (21) since at  $\beta_i=1$  and  $r_i=1$  the non-linear stress-strain relation (8) transforms into the Hooke's law assuming that  $E_{0_i}$  is the modulus of elasticity.

Eqs. (19), (20) and (21) should be solved with respect to  $\varepsilon_{C_1}$ ,  $\kappa_{y_1}$  and  $\kappa_{z_1}$  by using the MatLab computer program.

The complementary strain energy cumulated in the un-cracked beam portion is written as

$$U_U^* = (l - a) \sum_{i=1}^n \int_{y_{2i}}^{y_{2i+1}} \int_{-\frac{h}{2}}^{\frac{h}{2}} u_{0U_i}^* dy_2 dz_2, \quad (41)$$

where  $n$  is the number of the layers,  $y_{2i}$  and  $y_{2i+1}$  are the coordinates, respectively, of the left-hand and the right-hand lateral surfaces of the  $i$ -th layer,  $u_{0U_i}^*$  is the complementary strain energy density in the same layer,  $y_2$  and  $z_2$  are the centroidal axes of the beam cross-section.

Formula (10) is applied to obtain  $u_{0U_i}^*$ . For this purpose,  $E_i$ ,  $\beta_i$ ,  $r_i$  and  $\varepsilon$  are replaced with  $E_{U_i}$ ,  $\beta_{U_i}$ ,  $r_{U_i}$  and  $\varepsilon_U$ , respectively ( $E_{U_i}$ ,  $\beta_{U_i}$  and  $r_{U_i}$  are the distributions of the material properties in the  $i$ -th layer of the un-cracked beam portion,  $\varepsilon_U$  is the distribution of the longitudinal strains in the un-cracked beam portion). Formulae (11), (12), (13) and (14) are used to obtain  $E_{U_i}$ ,  $\beta_{U_i}$ ,  $r_{U_i}$  and  $\varepsilon_U$ , respectively. For this purpose,  $y_1$ ,  $y_{li}$ ,  $y_{li+1}$ ,  $z_1$ ,  $\varepsilon_{C_1}$ ,  $\kappa_{y_1}$  and  $\kappa_{z_1}$  are replaced with  $y_2$ ,  $y_{2i}$ ,

$y_{2i+1}$ ,  $z_2$ ,  $\varepsilon_{C_2}$ ,  $\kappa_{y_2}$  and  $\kappa_{z_2}$ , respectively ( $\varepsilon_{C_2}$  is the strain in the centre of the cross-section of the un-cracked beam portion,  $\kappa_{y_2}$  and  $\kappa_{z_2}$  are the curvatures of the un-cracked beam portion in  $x_2y_2$  and  $x_2z_2$  planes, respectively). Equilibrium Eqs. (19), (20) and (21) are used to determine  $\varepsilon_{C_2}$ ,  $\kappa_{y_2}$  and  $\kappa_{z_2}$ . For this purpose,  $N_1$ ,  $M_{y_1}$ ,  $M_{z_1}$ ,  $n_L$ ,  $\varepsilon_{C_1}$ ,  $\kappa_{y_1}$ ,  $\kappa_{z_1}$ ,  $y_{1i}$ ,  $y_{1i+1}$ ,  $\psi_i$ ,  $\gamma_i$ ,  $\theta_i$ ,  $\omega_i$ ,  $\varphi_i$ ,  $\phi_i$ ,  $\lambda_i$ ,  $\chi_i$ ,  $\nu_i$ ,  $\vartheta_i$ ,  $\mu_i$ ,  $\mu_i$ ,  $\zeta_i$ ,  $c_i$  and  $q_i$  are replaced, respectively, with  $N_2$ ,  $M_{y_2}$ ,  $M_{z_2}$ ,  $n$ ,  $\varepsilon_{C_2}$ ,  $\kappa_{y_2}$ ,  $\kappa_{z_2}$ ,  $y_{2i}$ ,  $y_{2i+1}$ ,  $\psi_{U_i}$ ,  $\gamma_{U_i}$ ,  $\theta_{U_i}$ ,  $\omega_{U_i}$ ,  $\varphi_{U_i}$ ,  $\phi_{U_i}$ ,  $\lambda_{U_i}$ ,  $\chi_{U_i}$ ,  $\nu_{U_i}$ ,  $\vartheta_{U_i}$ ,  $\mu_{U_i}$ ,  $\mu_{U_i}$ ,  $\zeta_{U_i}$ ,  $c_{U_i}$  and  $q_{U_i}$  in formulae (19)-(37), where  $N_2$  is the axial force in the un-cracked beam portion,  $M_{y_2}$  and  $M_{z_2}$  are the bending moments about  $y_2$  and  $z_2$  axes (apparently,  $N_2=0$ ,  $M_{y_2}=M$  and  $M_{z_2}=0$ ).

The strain energy in half of the beam is written as

$$U = U_L + U_U, \quad (42)$$

where  $U_L$  and  $U_U$  are the strain energies cumulated in the left-hand crack arm and the un-cracked beam portion, respectively.

The strain energy in the left-hand crack arm is expressed as

$$U_L = a \sum_{i=1}^{i=n_L} \int_{y_{1i}}^{y_{1i+1}} \int_{-\frac{h}{2}}^{\frac{h}{2}} u_{0L_i} dy_1 dz_1, \quad (43)$$

where the strain energy density,  $u_{0L_i}$ , in the  $i$ -th layer of the left-hand crack arm is obtained by (9).

The strain energy cumulated in the un-cracked beam portion is written as

$$U_U = (l-a) \sum_{i=1}^{i=n} \int_{y_{2i}}^{y_{2i+1}} \int_{-\frac{h}{2}}^{\frac{h}{2}} u_{0U_i} dy_2 dz_2, \quad (44)$$

where  $u_{0U_i}$  is determined by (9). For this purpose,  $E_i$ ,  $\beta_i$ ,  $r_i$  and  $\varepsilon$  are replaced with  $E_{U_i}$ ,  $\beta_{U_i}$ ,  $r_{U_i}$  and  $\varepsilon_U$ , respectively.

The expression obtained by substituting of (3), (4), (5), (41), (42), (43) and (44) in (2) is doubled in view of the symmetry (Fig. 1). In this manner, one arrives at

$$G = 2 \left[ \frac{M}{h} \frac{\partial}{\partial M} \left( \sum_{i=1}^{i=n_L} \int_{y_{1i}}^{y_{1i+1}} \int_{-\frac{h}{2}}^{\frac{h}{2}} u_{0L_i}^* dy_1 dz_1 - \sum_{i=1}^{i=n} \int_{y_{2i}}^{y_{2i+1}} \int_{-\frac{h}{2}}^{\frac{h}{2}} u_{0U_i}^* dy_2 dz_2 \right) - \frac{1}{h} \left( \sum_{i=1}^{i=n_L} \int_{y_{1i}}^{y_{1i+1}} \int_{-\frac{h}{2}}^{\frac{h}{2}} u_{0L_i} dy_1 dz_1 - \sum_{i=1}^{i=n} \int_{y_{2i}}^{y_{2i+1}} \int_{-\frac{h}{2}}^{\frac{h}{2}} u_{0U_i} dy_2 dz_2 \right) \right]. \quad (45)$$

The integration in (45) should be performed by the MatLab computer program. The derivative,

$\frac{\partial}{\partial M}(\dots)$ , in (45) should be determined numerically by the MatLab computer program.

The strain energy release rate (45) is verified by the  $J$ -integral approach (Rice 1968, Broek 1986). The integration is carried-out along the integration contour,  $\Gamma$ , shown by a dashed line in Fig. 1. It is obvious that the  $J$ -integral has non-zero values only in segments,  $\Gamma_1$  and  $\Gamma_2$ , of the integration contour, where  $\Gamma_1$  coincides with the cross-section of the left-hand crack arm in the beam mid-span,  $\Gamma_2$  coincides with the end section of the beam (Fig. 1). Therefore, the  $J$ -integral is obtained as

$$J = J_{\Gamma_1} + J_{\Gamma_2}, \quad (46)$$

where  $J_{\Gamma_1}$  and  $J_{\Gamma_2}$  are the values of the integral in segments  $\Gamma_1$  and  $\Gamma_2$ , respectively.

The  $J$ -integral in segment,  $\Gamma_1$ , is written as

$$J_{\Gamma_1} = \sum_{i=1}^{i=n_L} \int_{y_{1i}}^{y_{1i+1}} \left[ u_{0L_i} \cos \alpha - \left( p_{xi} \frac{\partial u}{\partial x} + p_{yi} \frac{\partial v}{\partial x} \right) \right] ds, \quad (47)$$

where  $\alpha$  is the angle between the outwards normal vector to the contour of integration and the crack direction,  $p_{xi}$  and  $p_{yi}$  are the components of stress vector in the  $i$ -th layer of the left-hand crack arm,  $u$  and  $v$  are the components of displacement vector with respect to the coordinate system  $xy$  ( $x$  is directed along the delamination crack),  $ds$  is a differential element along the contour.

The components of  $J_{\Gamma_1}$  are determined as

$$p_{xi} = -\sigma_i, \quad (48)$$

$$p_{yi} = 0, \quad (49)$$

$$ds = dy_1, \quad (50)$$

$$\cos \alpha = -1, \quad (51)$$

where  $\sigma_i$  is expressed as a function of  $\varepsilon$  by (8). The following formula from Mechanics of materials is applied to obtain the partial derivative,  $\partial u / \partial x$ , in (47)

$$\frac{\partial u}{\partial x} = \varepsilon = \varepsilon_{C_1} + \kappa_{y_1} y_1 + \kappa_{z_1} z_1. \quad (52)$$

The  $J$ -integral in segment,  $\Gamma_2$ , is expressed as

$$J_{\Gamma_2} = \sum_{i=1}^{i=n} \int_{y_{2i}}^{y_{2i+1}} \left[ u_{0U_i} \cos \alpha_U - \left( p_{xU_i} \frac{\partial u}{\partial x_U} + p_{yU_i} \frac{\partial v}{\partial x_U} \right) \right] ds_U, \quad (53)$$

where

$$p_{xU_i} = \sigma_i, \quad (54)$$

$$p_{yU_i} = 0, \quad (55)$$

$$ds_U = -dy_2, \quad (56)$$

$$\cos \alpha_U = 1, \quad (57)$$

$$\frac{\partial u}{\partial x_U} = \varepsilon_{C_2} + \kappa_{y_2} y_2 + \kappa_{z_2} z_2. \quad (58)$$

The stress,  $\sigma_i$ , in (54) is obtained by (8). For this purpose,  $E_i$ ,  $\beta_i$ ,  $r_i$  and  $\varepsilon$  are replaced with  $E_{U_i}$ ,  $\beta_{U_i}$ ,  $r_{U_i}$  and  $\varepsilon_U$ , respectively.

The average value of the  $J$ -integral along the delamination crack front is written as

$$J_{av} = \frac{1}{h} \int_{-\frac{h}{2}}^{\frac{h}{2}} J dz_1. \quad (59)$$

By substituting of (46), (47) and (53) in (59) and doubling of the expression obtained in view of the symmetry (Fig. 1), one arrives at

$$\begin{aligned} J_{av} = & \frac{2}{h} \sum_{i=1}^{i=n_l} \int_{-\frac{h}{2}}^{\frac{h}{2}} \int_{y_{1i}}^{y_{1i+1}} \left[ u_{0L_i} \cos \alpha - \left( p_{xi} \frac{\partial u}{\partial x} + p_{yi} \frac{\partial v}{\partial x} \right) \right] ds dz_1 + \\ & + \frac{2}{h} \sum_{i=1}^{i=n} \int_{-\frac{h}{2}}^{\frac{h}{2}} \int_{y_{2i}}^{y_{2i+1}} \left[ u_{0U_i} \cos \alpha_U - \left( p_{xU_i} \frac{\partial u}{\partial x_U} + p_{yU_i} \frac{\partial v}{\partial x_U} \right) \right] ds_U dz_1. \end{aligned} \quad (60)$$

The MatLab computer program should be used to perform the integration in (60). It should be noted that the  $J$ -integral values obtained by (60) match the strain energy release rates found by (45). This fact is a verification of the delamination fracture analysis of the multilayered functionally graded SSB configuration developed in the present paper.

### 3. Parametric analysis

A parametric analysis of the delamination fracture in the multilayered functionally graded SSB configuration is performed. Two three-layered functionally graded beams are investigated (Fig. 4). A delamination crack is located between layers 2 and 3 in the beam shown in Fig. 4(a). A beam with a delamination between layers 1 and 2 is also analyzed (Fig. 4(b)). The width of each layer is  $t$  (Fig. 4). The external loading consists of two moments,  $M$ , applied at the ends of the beams. It is

assumed that  $t=0.003$  m,  $h=0.014$  m and  $M=40$  Nm. The influence of the material gradients of  $E_i$ ,  $\beta_i$  and  $r_i$  in both width and height directions of layer 1 on the delamination fracture behaviour are investigated.

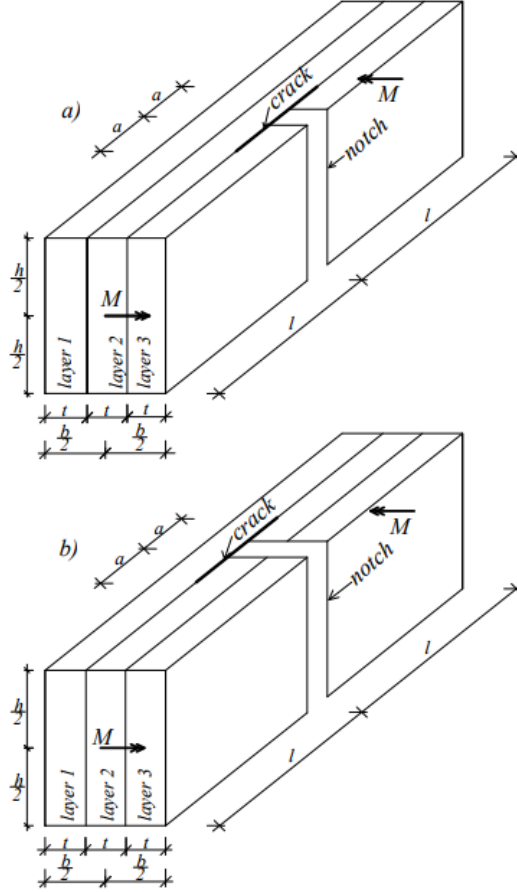


Fig. 4 Two three-layered functionally graded SSB configurations

For this purpose, calculations of the strain energy release rate are carried-out by applying formula (45). The strain energy release rate is presented in non-dimensional form by using the formula,  $G_N = G / (E_0 h)$ . The effect of material property,  $f_{E_1}$ , on the strain energy release rate for both three-layered SSB configurations (Fig. 4) is illustrated in Fig. 5. It is assumed that  $f_{E_2} = 0.8$ ,  $f_{E_3} = 1.2$ ,  $g_{E_1} = 0.6$ ,  $g_{E_2} = 0.9$ ,  $g_{E_3} = 1.1$ ,  $E_{0_2} / E_{0_1} = 0.8$ ,  $E_{0_3} / E_{0_1} = 1.6$ ,  $f_{\beta_1} = 0.4$ ,  $f_{\beta_2} = 0.2$ ,  $f_{\beta_3} = 0.5$ ,  $g_{\beta_1} = 0.3$ ,  $g_{\beta_2} = 0.4$ ,  $g_{\beta_3} = 0.6$ ,  $\beta_{0_1} = 0.3$ ,  $\beta_{0_2} = 0.6$ ,  $\beta_{0_3} = 0.4$ ,  $f_{r_1} = 0.1$ ,  $f_{r_2} = 0.3$ ,  $f_{r_3} = 0.2$ ,  $g_{r_1} = 0.2$ ,  $g_{r_2} = 0.1$ ,  $g_{r_3} = 0.3$ ,  $r_{0_1} = 0.4$ ,  $r_{0_2} = 0.1$  and  $r_{0_3} = 0.5$ .

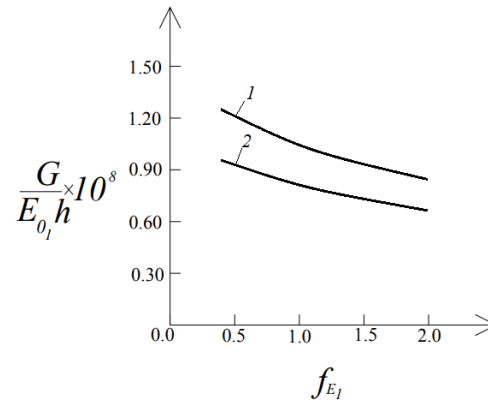


Fig. 5 The strain energy release rate in non-dimensional form plotted against  $f_{E_1}$  for a delamination crack located between layers 2 and 3 (curve 1) and for a delamination crack located between layers 1 and 2 (curve 2)

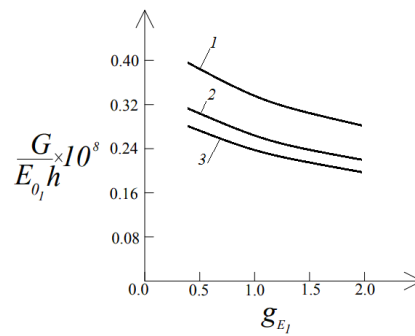


Fig. 6 The strain energy release rate in non-dimensional form plotted against  $g_{E_1}$  at non-linear behaviour of the functionally graded material (curve 1) and linear-elastic behaviour of the functionally graded material (curve 2) for the three-layered SSB configuration shown in Fig. 4(a)

The curves in Fig. 5 indicate that the strain energy release rate decreases with increasing of  $f_{E_1}$ . This finding is attributed to the fact that the beam stiffness increases when  $f_{E_1}$  increases. One can observe also in Fig. 5 that the strain energy release rate increases when the crack location is changed from this shown in Fig. 4(a) to that in Fig. 4(b). This behaviour is due to the decrease of the left-hand crack arm stiffness.

The influence of material property,  $g_{E_1}$ , on the fracture behaviour is evaluated too. The beam configuration shown in Fig. 4(a) is analyzed. The strain energy release rate in non-dimensional form is plotted in against  $g_{E_1}$  Fig. 6. It can be observed in Fig. 6 that the strain energy release rate decreases with increasing of  $g_{E_1}$ . The effect of the non-linear mechanical behaviour of the functionally graded material on the strain energy release rate is also illustrated in Fig. 6. For this purpose, the strain energy release rate obtained assuming linear-elastic behaviour of the functionally graded material is plotted in non-dimensional form in Fig. 6 for comparison with the non-linear solution. It should be mentioned that the linear-elastic solution for the strain energy

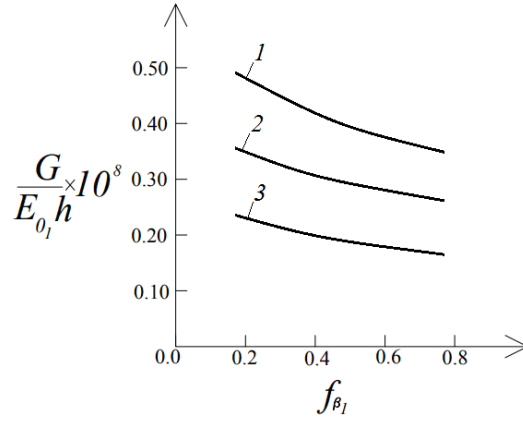


Fig. 7 The strain energy release rate in non-dimensional form plotted against  $f_{\beta_1}$  at  $g_{\beta_1} = 0.2$  (curve 1),  $g_{\beta_1} = 0.4$  (curve 2) and  $g_{\beta_1} = 0.6$  (curve 3) for the three-layered SSB configuration shown in Fig. 4(a)

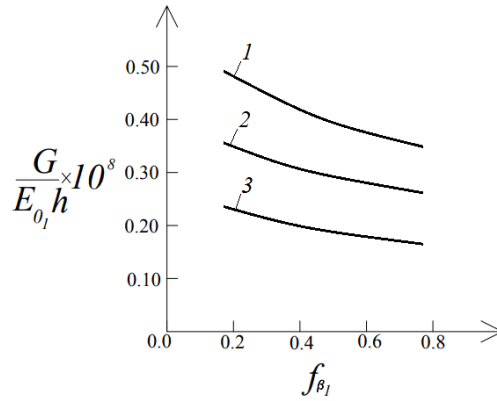


Fig. 8 The strain energy release rate in non-dimensional form plotted against  $f_{\beta_1}$  at  $g_{\beta_1} = 0.1$  (curve 1),  $g_{\beta_1} = 0.2$  (curve 2) and  $g_{\beta_1} = 0.3$  (curve 3) for the three-layered SSB configuration shown in Fig. 4(a)

release rate is derived by substituting of  $\beta_{0_i} = 1$ ,  $f_{\beta_i} = 0$ ,  $g_{\beta_i} = 0$ ,  $r_{0_i} = 1$ ,  $f_{r_i} = 0$  and  $g_{r_i} = 0$  in (45). One can observe in Fig. 6 that the material non-linearity leads to increase of the strain energy release rate.

The influence of material properties,  $f_{\beta_1}$  and  $g_{\beta_1}$ , on the strain energy release rate in the three-layered functionally graded SSB configuration shown in Fig. 4(a) is also explored. The strain energy release rate in non-dimensional form is presented as a function of  $f_{\beta_1}$  in Fig. 7 at three values of  $g_{\beta_1}$ . The diagrams in Fig. 7 show that the strain energy release rate decreases with increasing of  $f_{\beta_1}$  and  $g_{\beta_1}$ .

The effects of material properties,  $f_{r_1}$  and  $g_{r_1}$ , on the delamination fracture behaviour are also

evaluated. The SSB configuration shown in Fig. 4(a) is analyzed. The strain energy release rate in non-dimensional form is plotted against  $f_{r_1}$  in Fig. 8 at three values of  $g_{r_1}$ . It can be observed in Fig. 8 that the strain energy release rate decreases with increasing of  $f_{r_1}$  and  $g_{r_1}$ .

#### 4. Conclusions

Analyses of the delamination fracture in the multilayered functionally graded SSB configurations which exhibit non-linear mechanical behaviour of the material are developed. The SSB is made of an arbitrary number of adhesively bonded lengthwise vertical layers which have individual widths and material properties.

The non-linear mechanical behaviour of the material in each layer is described by a non-linear stress-strain relation that involves three material properties.

It is assumed that the three material properties are functionally graded in both width and height directions in each layer. Sine laws are used to describe the continuous variations of the material properties in the layers (the material properties are distributed non-symmetric with respect to the centroidal axes of the beam cross-section). Fracture is studied in terms of the strain energy release rate by analyzing the balance of the energy. The solution derived is compared with the  $J$ -integral for verification. The effects of gradients of the material properties along the width and height of the layers on the delamination fracture are investigated. The influence of delamination crack location along the width of the beam cross-section is analyzed too. The main findings of the present delamination fracture study can be summarized as follows:

- The strain energy release rate decreases with increasing of  $f_{E_1}$  and  $g_{E_1}$ .
- The increase of  $f_{\beta_1}$  and  $g_{\beta_1}$  leads to decrease of the strain energy release rate.
- The strain energy release rate decreases with increasing of  $f_{r_1}$  and  $g_{r_1}$ .
- The strain energy release rate decreases when the width of the cross-section of the left-hand crack arm increases.

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