Bending analysis of advanced composite plates using a new quasi 3D plate theory

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Abstract. In this paper, a refined higher-order shear deformation theory including the stretching effect is developed for the analysis of bending analysis of the simply supported functionally graded (FG) sandwich plates resting on elastic foundation. This theory has only five unknowns, which is even less than the other shear and normal deformation theories. The theory presented is variationally consistent, without the shear correction factor. The present one has a new displacement field which introduces undetermined integral variables. Equations of motion are obtained by utilizing the Hamilton's principles and solved via Navier's procedure. The convergence and the validation of the proposed theoretical numerical model are performed to demonstrate the efficacy of the model.

Keywords: FG sandwich plates; new plate theory; bending; stretching effect; analytical modeling

1. Introduction

Sandwich structures are one of the most functional forms of advanced composite structures developed by engineers. The main concept of sandwich composite structures is its high specific strength and bending stiffness to weight ratio (Belouettar et al. 2009). With its many advantages, composite sandwich structures have been widely used in a variety of engineering application including automotive, aerospace, mechanical, ships and other industrial applications (Vel et al. 2005). This composite material also draws a lot of interest in the construction industry and is now beginning to be in use for civil engineering projects as industrial buildings, vehicular bridges, solar power stations, nuclear reactor structures and petrochemical structures (Bennoun et al. 2016, Bounouara et al. 2016, Kolahchi et al. 2017a, b, Kolahchi 2017, El-Haina et al. 2017, Amar et al. 2017). Sandwich constructions consist of two outer strong layers and an inner relatively thick, lightweight core material (Vinson 2001). Sandwich structure has become even more attractive to the introduction of advanced composite materials for the face sheets like functionally graded ceramic-metal materials (Wang and Shen 2012). The considerable advantages

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offered by functionally graded materials (FGMs) over conventional materials are to eliminates the interface problems of conventional composite materials and thus the stress distribution becomes smooth (Li et al. 2008). Subsequently, a number of studies have been performed to analyze the static, vibration, and buckling of functionally graded structures due to the increased relevance of the FGMs structural components in the design of engineering structures (Tounsi et al. 2013, Kar and Panda 2014, 2015, Yaghoobi et al. 2014, Swaminathan and Naveenkumar 2014, Bousahla et al. 2014, Sofiyev and Kuruoglu 2015a, Akbaş 2015, Attia et al. 2015, Darilmaz 2015, Darilmaz et al. 2015, Bourada et al. 2015, Kolahchi et al. 2015, Mahi et al. 2015, Sofiyev and Osmancelebioglu 2017, Brischetto 2017, Benadouda et al. 2017, Khetir et al. 2017, Menasria et al. 2017). Plates supported elastic foundations are generally encountered in many engineering applications, such as bottom plates of hydraulic structures and surface plates of the airport (Ke-rang 1990). From the literature review, it is found that there are many studies of plates supported by elastic foundation and these studies have attracted the attention of many investigators. Al-Hosani et al. (1999) proposed a fundamental solution and boundary integral equations for thick Reissner plates resting on a Winkler elastic foundation by considering the effect of transverse normal stresses resulting from the foundation reaction on the plate surface. Darilmaz (2009) proposed a four-node hybrid stress element for analysing arbitrarily shaped plates resting on a two parameter elastic foundation. Baltacioglu et al. (2011) proposed a method of discrete

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singular convolution for the large deflection analysis of laminated composite plates resting on nonlinear elastic foundations. A few researchers have utilized classical plate theory (CPT) to studies vibration and static behavior of thin functionally graded (FG) plates. He et al. (2001) reported the finite element formulation based on thin plate theory to control the shape and vibration of FGM plate with integrated piezoelectric sensors and actuators under mechanical load. Woo et al. (2006) provided an analytical solution for the nonlinear free vibration behavior of FG square thin plates using the von Karman theory. Abrate (2008) has presented result of free vibration of simply supported and clamped rectangular thin plates using the CPT. Zhang and Zhou (2008) studies the free vibration, buckling and on the basis of the physical neutral surface. The classical plate theory ignores the transverse shear strain and is suitable only to studies thin plates. However, it is not appropriate for the moderately thick and thick plates, which require that the transverse and normal strain should be taken into account. First-order shear deformation theory considers the transverse shear deformation effects and gives acceptable results for thick and thin plates, but needs a shear correction factor which is hard to find as it depends on the geometries, material properties and boundary conditions of each problem (Ferreira et al. 2009). Liu and Liew (1999) analyzed free vibration of rectangular plates with mixed boundary conditions using the first-order shear deformation theory (FSDT). The nonlinear static and dynamic responses of functionally graded ceramic-metal plates using the FSDT and the von Karman strain were examined by Praveen and Reddy (Praveen and Reddy 1998). Najafov et al. (2014) investigated the stability of exponentially graded (EG) cylindrical shells with shear stresses on a Pasternak foundation based on the first-order shear deformation theory (FSDT). Sofiyev et al. (2015b) studied the free vibration of sandwich cylindrical shells covered by functionally graded coatings and resting on the Pasternak elastic foundation considering combined influences of shear stresses and rotary inertia are examined. Sofiyev et al. (2016) analyzed the vibration and stability of axially loaded functionally graded (FG) sandwich cylindrical shells with and without shear stresses and rotary inertia resting Pasternak foundation. Based on the orthotropic Mindlin plate, Kolahchi et al. (2016) investigated the temperature-dependent nonlinear dynamic stability for a functionally graded CNT reinforced viscoplate resting on an orthotropic elastomeric foundation. Chen et al. (2017) investigated the thermal buckling and vibration of initially stressed sandwich plates with functionally graded material (FGM) face sheets, including the effects of transverse shear deformation and rotary inertia. The higherorder shear deformation theories (HSDTs) have been developed and do not require any shear correction factor. These theories include higher-order terms in the approximation of the in-plane displacement fields and satisfy zero shear stress conditions at top and bottom surfaces of plates. Reddy (2000) has presented result of the static behavior of functionally graded rectangular plates based on a third-order shear deformation plate theory. Cheng and Batra (2000) derived the field equations for a simply supported functionally graded plate by utilizing the first-order shear deformation theory or the third-order shear deformation theory and simplified them for a simply supported polygonal plate to that of an equivalent homogeneous Kirchhoff plate. Yaghoobi and Yaghoobi (2013) investigated the buckling analysis of symmetric sandwich plates with FG face sheets resting on an elastic foundation based on the first-order shear deformation plate theory and under to mechanical, thermal and thermomechanical loads. Ferreira et al. (2006) studied the free vibration of functionally graded plates based on the first and the third-order shear deformation plate theories using the Mori-Tanaka homogenization method and the global collocation method with multiquadratic radial basis functions. Bellifa et al. (2016) studied the bending and the free vibration of functionally graded plates using a novel simple first-order shear deformation plate theory based on neutral surface position. Some studies on response of FG sandwich plates have been carried out using higher-order shear deformation theories. Zenkour (2005a, b) investigated bending, vibration and buckling problem of sandwich plates with FG faces and homogeneous hardcore using different shear deformation theories. Ait Amar Meziane et al. (2014) analyzed the buckling and free vibration of exponentially graded sandwich plates under various boundary conditions using an efficient and simple refined theory. Benyoucef et al. (2010) examined the static response of simply supported functionally graded plates resting on an elastic foundation using a new hyperbolic displacement model. Zenkour and Sobhy (2010) studied the critical buckling temperature for FGM sandwich plates using a sinusoidal shear deformation plate theory to derive the appropriate elastic stability equations. Ebrahimi and Habibi (2016) utilized the finite element method is to predict the deflection and vibration of porous FG plates made within the framework of the third order shear deformation plate theory. Recently, Tounsi and his co-workers (Hadji et al. 2011, Houari et al. 2011, Merdaci et al. 2011, Bouderba et al. 2013, Zidi et al. 2014, Ait Yahia et al. 2015, Boukhari et al. 2016, Hadj Henni et al. 2017) developed a new refined and robust plate theory for bending response, buckling, vibration and wave propagation of simply supported FG plate with only four unknown functions. In Houari et al. (2016) and Tounsi et al. (2016) a new simple shear deformation theory for the bending and free vibration response of FG plates with only three unknown functions was developed. As opposed to five or even greater numbers in the case of other higher shear deformation theories, this theory is variationally consistent, does not require shear correction factor, and accounts for parabolic distribution of the transverse shear strains, and satisfies the zero traction on the surfaces of the plate without using shear correction factor. Most recently, Tounsi and his co-workers (Hebali et al. 2016, Merdaci et al. 2016, Krenich et al. 2017, Meftah et al. 2017) developed another novel refined plate theory for mechanical behaviour of simply supported plate with only four unknown. These theories have a new displacement field which introduces undetermined integral variables. It should be noted that the thickness stretching effect is ignored in these new fourvariable plate theories and other hear deformation plates

theories. The importance of the thickness stretching effect in FG plates has been pointed and discussed out in the work of Carrera et al. (2011) using appropriate finite element approximations. Quasi-3D theories are higher order shear deformation theories (HSDTs) with higher-order variations through the thickness for the transverse displacement. There are many papers proposed in the literature concerned with investigation of the different behaviors of the FGM structures by using the quasi-3D theories. Kant and Swaminathan (2002) proposed a quasi-3D theory with all displacement components expanded as a cubic variation through the thickness for static response of laminated composite and sandwich plates. Neves et al. (2011) and Neves et al. (2012a, b) have presented an original hyperbolic sine shear deformation theory for the bending and free vibration analysis of FG plates. Houari et al. (2013) developed a theory for the thermoelastic bending analysis of FGM sandwich plates. Bessaim et al. (2013) presented a theory for the bending and free vibration analysis of sandwich plates. Belabed et al. (2014) have determined the bending and free vibration response for FG plate. Hebali et al. (2014) analyzed the static and free vibration analysis of functionally graded plates. Hamidi et al. (2015) have proposed a sinusoidal plate theory with 5unknowns and stretching effect for thermomechanical bending of functionally graded sandwich plates. Draiche et al. (2016) have presented a refined theory with stretching effect for the flexure analysis of laminated composite plates. Sekkal et al. (2017) developed a new quasi-3D HSDT for buckling and vibration of FG plate. Recently, Abualnour et al. (2018) proposed a novel quasi-3D trigonometric plate theory for free vibration analysis of advanced composite plates. Benchohra et al. (2018) presented a new quasi-3D sinusoidal shear deformation theory for FG plates.

This paper aims to improve the plate theory developed by Tounsi and his co-workers (Hebali et al. 2016, Merdaci et al. 2016, Krenich et al. 2017, Meftah et al. 2017) by including the so-called stretching effect. Using the proposed theory, both static behavior of FGM sandwich plates resting on two-parameter elastic foundations are investigated. This theory has only five unknowns, which is even less than the other Quasi-3D theories. The most interesting feature of this theory is that it accounts for a hyperbolic variation of the transverse shear strains across the thickness and satisfies the zero traction boundary conditions on the top and bottom surfaces of the plate without using shear correction factors. The present one has a new displacement field which introduces undetermined integral variables. Analytical solutions are obtained for FGM sandwich plate, and accuracy is verified by comparing the obtained results with those reported in the literature.

2. Theoretical formulation

2.1 Geometrical configuration

A solid rectangular FG sandwich plate with uniform thickness with uniform thickness h, length a, and width b is considered in the present study (Fig. 1). The rectangular

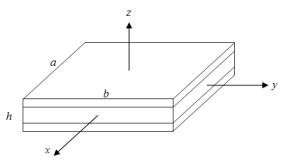


Fig. 1 Geometry of rectangular FGM sandwich plate with uniform thickness in the rectangular Cartesian coordinates

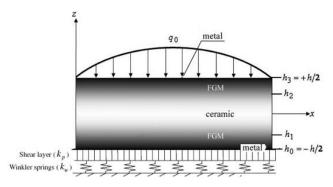


Fig. 2 The material variation along the thickness of the FGM sandwich plate

Cartesian coordinate system (x, y, z) is taken such that the x, y plane (z = 0) coincides with the mid-plane of the sandwich plate. The top and bottom faces of the plate are at $z = \pm h/2$, and the edges of the plate are parallel to axes x and y. The sandwich plate is composed of three elastic layers, namely, "Layer 1", "Layer 2", and "Layer 3" from bottom to top of the plate. The vertical ordinates of the bottom, the two interfaces, and the top are denoted by $h_0 = -h/2$, h_1 , h_2 , h_3 = h / 2, respectively. In order to study the effect of the thickness variation of the three layers on displacements and stresses, a simple notation is used in all the numerical examples considered. For example, a notation of 1-2-1 (i.e., bottom face-core-top face thickness) is used to represent that the top and bottom face sheets have same thickness, whereas the core thickness is twice the bottom/top thickness. Other cases, viz. 1-0-1, 1-1-1, 2-1-2, 3-1-3 and 2-2-1, are also considered to realize the various numerical examples.

The face layers of the sandwich plate are made of an isotropic material with material properties varying smoothly in the z direction only. The core layer is made of an isotropic homogeneous material, as shown in Fig. 2.

2.2 Materials proprieties

A simple power law in terms of the volume fraction of the ceramic phase is considered

$$V^{(1)} = \left(\frac{z - h_0}{h_1 - h_0}\right)^p, \quad z \in [h_0, h_1]$$
 (1a)

$$V^{(2)} = 1, \quad z \in [h_1, h_2]$$
 (1b)

$$V^{(3)} = \left(\frac{z - h_3}{h_2 - h_3}\right)^p, \quad z \in [h_2, h_3]$$
 (1c)

where $V^{(n)}$, (n = 1, 2, 3) represents the volume fraction function of layer n; p is the volume fraction index $(0 \le p \le +\infty)$, which control the material distribution in the thickness direction.

The effective material properties, like Young's modulus E, and Poisson's ratio v, can be mathematically expressed by the rule of mixture as (Marur 1999, Bourada $et\ al.\ 2011$, Houari $et\ al.\ 2013$, Bellifa $et\ al.\ 2017$, Besseghier $et\ al.\ 2017$)

$$P^{(n)}(z) = P_2 + (P_1 - P_2)V^{(n)}$$
 (2)

where $P^{(n)}$ is the effective material property of FGM of layer n. P_1 and P_2 are the properties of the top and bottom faces of layer 1, respectively, and vice versa for layer 3 depending on the volume fraction $V^{(n)}$, (n = 1, 2, 3). For simplicity, Poisson's ratio of plate is assumed to be constant in this study for that the effect of Poisson's ratio on the deformation is much less than that of Young's modulus (Delale and Erdogan 1983).

2.3 Constitutive equations

For elastic and isotropic FGMs, the constitutive relations can be written as

$$\begin{cases}
\sigma_{x} \\
\sigma_{y} \\
\sigma_{z} \\
\tau_{xy}
\end{cases}^{(n)} =
\begin{bmatrix}
Q_{11} & Q_{12} & Q_{13} & 0 \\
Q_{12} & Q_{22} & Q_{23} & 0 \\
Q_{13} & Q_{23} & Q_{33} & 0 \\
0 & 0 & 0 & Q_{66}
\end{bmatrix}^{(n)}
\begin{cases}
\varepsilon_{x} \\
\varepsilon_{y} \\
\varepsilon_{z} \\
\gamma_{xy}
\end{cases}^{(n)}$$
(3a)

and

where $(\sigma_x, \sigma_y, \sigma_z, \tau_{xy}, \tau_{yz}, \tau_{yx})$ and $(\varepsilon_x, \varepsilon_y, \varepsilon_z, \gamma_{xy}, \gamma_{yz}, \gamma_{yx})$ are the stress and strain components, respectively. Using the material properties defined in Eq. (3), stiffness coefficients, Q_{ij} , can be expressed as

$$Q_{11} = Q_{22} = Q_{33} = \frac{(1-\nu)E(z)}{(1-2\nu)(1+\nu)},$$
 (4a)

$$Q_{12} = Q_{13} = Q_{23} = \frac{v(1-v)E(z)}{(1-2v)(1+v)},$$
 (4b)

$$Q_{44} = Q_{55} = Q_{66} = \frac{E(z)}{2(1+v)},$$
 (4c)

Based on the thick plate theory and including the effect of transverse normal stress (thickness stretching effect), the basic assumptions for the displacement field of the plate can be described as

$$u(x, y, z) = u_0(x, y, t) - z \frac{\partial w_0}{\partial x} + k_1 f(z) \int \theta(x, y) dx$$
 (5a)

$$v(x, y, z) = v_0(x, y, t) - z \frac{\partial w_0}{\partial y} + k_2 f(z) \int \theta(x, y) dy$$
 (5b)

$$w(x, y, z) = w_0(x, y, t) + g(z)\varphi_z(x, y)$$
 (5c)

The coefficients k_1 and k_2 depends on the geometry and the proposed theory of present study has a hyperbolic function in the form

$$f(z) = h \sinh\left(\frac{z}{h}\right) - z \cosh\left(\frac{1}{2}\right) \tag{6}$$

It can be observed that the kinematic in Eq. (5) uses only five unknowns (u_0 , v_0 , w_0 , θ and φ_z). Nonzero strains of the five variable plate model are expressed as follows

$$\begin{cases}
\varepsilon_{x} \\
\varepsilon_{y} \\
\gamma_{xy}
\end{cases} = \begin{cases}
\varepsilon_{x}^{0} \\
\varepsilon_{y}^{0} \\
\gamma_{xy}^{0}
\end{cases} + z \begin{cases}
k_{x}^{b} \\
k_{y}^{b} \\
k_{xy}^{b}
\end{cases} + f(z) \begin{cases}
k_{x}^{s} \\
k_{y}^{s} \\
k_{xy}^{s}
\end{cases} \tag{7a}$$

$$\begin{cases} \gamma_{yz} \\ \gamma_{yz} \end{cases} = g(z) \begin{cases} \gamma_{yz}^{0} \\ \gamma_{yz}^{0} \end{cases}$$
 (7b)

$$\varepsilon_z = g'(z)\,\varepsilon_z^0\tag{7c}$$

where

$$\begin{cases}
\mathcal{E}_{x}^{0} \\
\mathcal{E}_{y}^{0} \\
\mathcal{Y}_{xy}^{0}
\end{cases} = \begin{cases}
\frac{\partial u_{0}}{\partial x} \\
\frac{\partial v_{0}}{\partial x} \\
\frac{\partial u_{0}}{\partial y} + \frac{\partial v_{0}}{\partial x}
\end{cases},
\begin{cases}
k_{x}^{b} \\
k_{y}^{b} \\
k_{xy}^{b}
\end{cases} = \begin{cases}
-\frac{\partial^{2} w_{0}}{\partial x^{2}} \\
-\frac{\partial^{2} w_{0}}{\partial y^{2}} \\
-2\frac{\partial^{2} w_{0}}{\partial x \partial y}
\end{cases},$$

$$\begin{cases}
k_{x}^{s} \\
k_{y}^{s} \\
k_{y}^{s}
\end{cases} = \begin{cases}
k_{1}\theta \\
k_{2}\theta \\
k_{1}\frac{\partial}{\partial y}\int\theta \,dx + k_{2}\frac{\partial}{\partial x}\int\theta \,dy
\end{cases}$$
(8a)

$$\begin{cases}
\gamma_{yz}^{0} \\
\gamma_{xz}^{0}
\end{cases} = \begin{cases}
k_{2} \int \theta \, dy + \frac{\partial \varphi_{z}}{\partial y} \\
k_{1} \int \theta \, dx + \frac{\partial \varphi_{z}}{\partial x}
\end{cases}, \qquad \varepsilon_{z}^{0} = \varphi_{z} \tag{8b}$$

and

$$g'(z) = \frac{dg(z)}{dz}$$
 (8c)

It can be observed from Eq. (7) that the transverse shear

strains $(\gamma_{xz}, \gamma_{yz})$ are equal to zero at the upper (z = h/2) and lower (z = -h/2) surfaces of the plate. A shear correction coefficient is, hence, not required.

The integrals used in the above equations shall be resolved by a Navier type procedure and can be expressed as follows

$$\frac{\partial}{\partial y} \int \theta \, dx = A' \frac{\partial^2 \theta}{\partial x \partial y}, \qquad \frac{\partial}{\partial x} \int \theta \, dy = B' \frac{\partial^2 \theta}{\partial x \partial y},$$

$$\int \theta \, dx = A' \frac{\partial \theta}{\partial x}, \qquad \int \theta \, dy = B' \frac{\partial \theta}{\partial y}$$
(9)

where the coefficients A' and B' are considered according to the type of solution employed, in this case via Navier method. Therefore, A', B', k_1 and k_2 are expressed as follows

$$A' = -\frac{1}{\alpha^2}, \quad B' = -\frac{1}{\beta^2}, \quad k_1 = \alpha^2, \quad k_2 = \beta^2$$
 (10)

where α and β are defined in Eq. (21).

2.4 Governing equations

The principle of virtual displacements is utilized for the bending problem of FG sandwich plates. The principle of virtual work in the present case yields (Reddy 2002, Al-Basyouni *et al.* 2015, Zemri *et al.* 2015, Ahouel *et al.* 2016, Saidi *et al.* 2016, Mouffoki *et al.* 2017, Zidi *et al.* 2017, Hachemi *et al.* 2017)

$$\delta U = \int_{-h/2}^{h/2} \int_{\Omega} \left[\sigma_{x} \delta \varepsilon_{x} + \sigma_{y} \delta \varepsilon_{y} + \sigma_{z} \delta \varepsilon_{z} + \tau_{xy} \delta \gamma_{xy} \right] d\Omega dz + \tau_{yz} \delta \gamma_{yz} + \tau_{xz} \delta \gamma_{xz} d\Omega dz - \int_{\Omega} (q - f_{e}) \delta w d\Omega = 0$$
(11)

where f_e is the density of reaction force of foundation. For the Pasternak foundation model

$$f_e = k_w w - k_s^1 \frac{\partial^2 w}{\partial x^2} - k_s^2 \frac{\partial^2 w}{\partial y^2}$$
 (12)

where k_w is the modulus of subgrade reaction (elastic coefficient of the foundation) and k_s^1 and k_s^2 are the shear moduli of the subgrade (shear layer foundation stiffness). If foundation is homogeneous and isotropic, we will get $k_s^1 = k_s^2 = k_s$. If the shear layer foundation stiffness is neglected, Pasternak foundation becomes a Winkler foundation.

Substituting Eqs. (4) and (7) into Eq. (12) and integrating through the thickness of the plate, Eq. (12) can be rewritten as

$$\int_{\Omega} \left[N_{x} \delta \varepsilon_{x}^{0} + N_{y} \delta \varepsilon_{y}^{0} + N_{z} \delta \varepsilon_{z}^{0} + N_{xy} \delta \gamma_{xy}^{0} \right]
+ M_{x}^{b} \delta k_{x}^{b} + M_{y}^{b} \delta k_{y}^{b} + M_{xy}^{b} \delta k_{xy}^{b} + M_{x}^{s} \delta k_{x}^{s}
+ M_{y}^{s} \delta k_{y}^{s} + M_{xy}^{s} \delta k_{xy}^{s} + S_{yz}^{s} \delta \gamma_{yz}^{s} + S_{xz}^{s} \delta \gamma_{xz}^{0} \right] d\Omega
- \int_{\Omega} \left(q - f_{e} \right) \delta w d\Omega = 0$$
(13)

where Ω is the top surface and the stress resultants N, M, and S are expressed by

$$(N_{i}, M_{i}^{b}, M_{i}^{s}) = \sum_{n=1}^{3} \int_{h_{n-1}}^{h_{n}} (1, z, f) \sigma_{i}^{(n)} dz \qquad (i = x, y, xy);$$

$$N_{z} = \int_{h_{n-1}}^{h_{n}} g'(z) \sigma_{z}^{(n)} dz \qquad (14a)$$

and

$$\left(S_{xz}^{s}, S_{yz}^{s}\right) = \int_{h_{n-1}}^{h_{n}} g\left(\tau_{xz}, \tau_{yz}\right)^{(n)} dz$$
 (14b)

where h_n and h_{n-1} are the top and bottom z-coordinates of the nth layer.

The governing equations of equilibrium can be derived from Eq. (11) by integrating the displacement gradients by parts and setting the coefficients δu_0 , δv_0 , δw_0 , $\delta \theta$, and $\delta \varphi_z$ zero, separately. The following equations of motion of associated with the present shear deformation theory are obtained as

$$\delta u_{0}: \frac{\partial N_{x}}{\partial x} + \frac{\partial N_{xy}}{\partial y} = 0$$

$$\delta v_{0}: \frac{\partial N_{xy}}{\partial x} + \frac{\partial N_{y}}{\partial y} = 0$$

$$\delta w_{0}: \frac{\partial^{2} M_{x}^{b}}{\partial x^{2}} + 2 \frac{\partial^{2} M_{xy}^{b}}{\partial x \partial y} + \frac{\partial^{2} M_{y}^{b}}{\partial y^{2}} - f_{e} + q = 0$$

$$\delta \theta: -k_{1} M_{x}^{s} - k_{2} M_{y}^{s} - (k_{1} A' + k_{2} B') \frac{\partial^{2} M_{xy}^{s}}{\partial x \partial y}$$

$$+k_{1} A' \frac{\partial S_{xz}^{s}}{\partial x} + k_{2} B' \frac{\partial S_{yz}^{s}}{\partial y} = 0$$

$$\delta \varphi_{z}: \frac{\partial S_{xz}^{s}}{\partial x} + \frac{\partial S_{yz}^{s}}{\partial y} - N_{z} + g(z)q = 0$$

$$(15)$$

Using Eq. (4) in Eq. (14), the stress resultants of a sandwich plate made up of three layers can be related to the total strains by

$$\begin{vmatrix} N_x \\ N_y \\ N_{xy} \\ M_x^b \\ M_x^b \\ M_x^s \\ M_x^s \\ M_y^s \\ N_z \end{vmatrix} = \begin{bmatrix} A_{11} & A_{12} & 0 & B_{11} & B_{12} & 0 & B_{11}^s & B_{12}^s & 0 & X_{13} \\ A_{12} & A_{22} & 0 & B_{12} & B_{22} & 0 & B_{12}^s & B_{22}^s & 0 & X_{23} \\ 0 & 0 & A_{66} & 0 & 0 & B_{66} & 0 & 0 & B_{66}^s & 0 \\ B_{11} & B_{12} & 0 & D_{11} & D_{12} & 0 & D_{11}^s & D_{12}^s & 0 & Y_{13} \\ B_{12} & B_{22} & 0 & D_{12} & D_{22} & 0 & D_{12}^s & D_{22}^s & 0 & Y_{23} \\ 0 & 0 & B_{66} & 0 & 0 & D_{11} & 0 & 0 & D_{66}^s & 0 \\ B_{11} & B_{12} & 0 & D_{11}^s & D_{12}^s & 0 & H_{11}^s & H_{12}^s & 0 & Y_{13}^s \\ 0 & 0 & B_{66} & 0 & 0 & D_{11}^s & D_{12}^s & 0 & Y_{23}^s \\ B_{12} & B_{22}^s & 0 & D_{12}^s & D_{22}^s & 0 & H_{12}^s & H_{22}^s & 0 & Y_{23}^s \\ 0 & 0 & B_{66}^s & 0 & 0 & D_{66}^s & 0 & 0 & H_{66}^s & 0 \\ X_{13} & X_{23} & 0 & Y_{13} & Y_{23} & 0 & Y_{13}^s & Y_{23}^s & 0 & Z_{33} \end{bmatrix} \begin{bmatrix} \mathcal{E}_x^0 \\ \mathcal{E}_y^0 \\ \mathcal{E}_y^0 \\ \mathcal{E}_y^0 \\ \mathcal{E}_x^0 \\ \mathcal{E}_y^0 \\ \mathcal{E}_z^0 \end{bmatrix}$$

$$\begin{cases}
S_{yz}^{s} \\
S_{xz}^{s}
\end{cases} = \begin{bmatrix}
A_{44}^{s} & 0 \\
0 & A_{55}^{s}
\end{bmatrix} \begin{bmatrix}
\gamma_{yz}^{0} \\
\gamma_{yz}^{0}
\end{bmatrix} \tag{16b}$$

where

$$\left(A_{ij}, A_{ij}^{s}, B_{ij}, D_{ij}, B_{ij}^{s}, D_{ij}^{s}, H_{ij}^{s}\right) = \int_{-h/2}^{h/2} C_{ij} \left(1, g^{2}(z), z, z^{2}, f(z), z f(z), f^{2}(z)\right) dz$$
(17a)

$$(X_{ij}, Y_{ij}, Y_{ij}^{s}, Z_{ij})$$

$$= \int_{-h/2}^{h/2} (1, z, f(z), g'(z)) g'(z) C_{ij} dz$$
(17b)

2.3 Equations of motion in terms of displacements

By substituting Eq. (16) into Eq. (15), the equilibrium equations can be expressed in terms of displacements (u_0 , v_0 , w_0 , θ and φ_z) as

$$A_{11}d_{11}u_{0} + A_{66} d_{22}u_{0} + (A_{12} + A_{66})d_{12}v_{0} + X_{13}d_{1}\varphi_{z} - B_{11}d_{111}w_{0} - (B_{12} + 2B_{66})d_{122}w_{0} + (B_{66}^{s}(k_{1}A' + k_{2}B'))d_{122}\theta + (B_{11}^{s}k_{1} + B_{12}^{s}k_{2})d_{1}\theta = 0,$$
(18a)

$$A_{22} d_{22}v_{0} + A_{66} d_{11}v_{0} + (A_{12} + A_{66}) d_{12}u_{0} + X_{23} d_{2} \varphi_{z} - B_{22} d_{222}w_{0} - (B_{12} + 2B_{66}) d_{112}w_{0} + (B_{66}^{s} (k_{1}A' + k_{2} B')) d_{112}\theta + (B_{52}^{s} k_{2} + B_{15}^{s} k_{1}) d_{2}\theta = 0,$$
(18b)

$$B_{11} d_{111} u_0 + (B_{12} + 2B_{66}) d_{122} u_0 + (B_{12} + 2B_{66}) d_{112} v_0$$

$$+ B_{22} d_{222} v_0 + Y_{13} d_{11} \varphi_z + Y_{23} d_{22} \varphi_z - D_{11} d_{1111} w_0$$

$$- 2 (D_{12} + 2D_{66}) d_{1122} w_0 - D_{22} d_{2222} w_0$$

$$+ (D_{11}^s k_1 + D_{12}^s k_2) d_{11} \theta$$

$$+ 2 (D_{66}^s (k_1 A' + k_2 B')) d_{1122} \theta$$

$$+ (D_{12}^s k_1 + D_{22}^s k_2) d_{22} \theta - f_e + q = 0$$

$$(18c)$$

$$-\left(B_{11}^{s}k_{1}+B_{12}^{s}k_{2}\right)d_{1}u_{0}-\left(B_{66}^{s}\left(k_{1}A^{'}+k_{2}B^{'}\right)\right)d_{122}u_{0}\\-\left(B_{66}^{s}\left(k_{1}A^{'}+k_{2}B^{'}\right)\right)d_{112}v_{0}-\left(B_{12}^{s}k_{1}+B_{22}^{s}k_{2}\right)d_{2}v_{0}\\-k_{1}Y_{13}^{s}\theta_{z}-k_{2}Y_{23}^{s}\theta_{z}+\left(D_{11}^{s}k_{1}+D_{12}^{s}k_{2}\right)d_{11}w_{0}\\+2\left(D_{66}^{s}\left(k_{1}A^{'}+k_{2}B^{'}\right)\right)d_{1122}w_{0}\\+\left(D_{12}^{s}k_{1}+D_{22}^{s}k_{2}\right)d_{22}w_{0}\\+\left(B_{12}^{s}k_{1}+B_{22}^{s}k_{2}\right)d_{22}w_{0}\\-\left(\left(k_{1}A^{'}+k_{2}B^{'}\right)^{2}H_{66}^{s}\right)d_{1122}\theta+A_{44}^{s}\left(k_{2}B^{'}\right)^{2}d_{22}\theta\\+A_{55}^{s}\left(k_{1}A^{'}\right)^{2}d_{11}\theta+A_{44}^{s}\left(k_{2}B^{'}\right)d_{22}\varphi_{z}\\+A_{55}^{s}\left(k_{1}A^{'}\right)d_{11}\varphi_{z}=0$$

$$(18d)$$

$$\begin{split} X_{13}d_{1}u_{0} + X_{23} d_{2}u_{0} + Z_{33}\varphi_{z} + Y_{13} d_{11}w_{0} + Y_{23} d_{22}w_{0} \\ + A_{44}^{s} \left(k_{2}B^{'}\right)d_{22}\theta + A_{55}^{s} \left(k_{1}A^{'}\right)d_{11}\theta \\ + A_{44}^{s} d_{22}\varphi_{z} + A_{55}^{s} d_{11}\varphi_{z} = 0, \end{split} \tag{18e}$$

where d_{ij} , d_{ijl} and d_{ijlm} are the following differential operators

$$d_{ij} = \frac{\partial^{2}}{\partial x_{i} \partial x_{j}}, \qquad d_{ijl} = \frac{\partial^{3}}{\partial x_{i} \partial x_{j} \partial x_{l}},$$

$$d_{ijlm} = \frac{\partial^{4}}{\partial x_{i} \partial x_{i} \partial x_{i} \partial x_{m}}, \quad d_{i} = \frac{\partial}{\partial x_{i}}, \qquad (i, j, l, m = 1, 2).$$
(19)

3. Analytical solution of simply supported FG sandwich plate

In this work, we are concerned with the exact solutions of Eq. (18) for a simply supported nanoplate. Using the Navier solution procedure, the following expressions of displacements $(u_0, v_0, w_0, \theta \text{ and } \varphi_z)$ are taken

$$\begin{cases}
 u_0 \\
 v_0 \\
 \theta \\
 \varphi_z
\end{cases} = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} \begin{cases}
 U_{mn} \cos(\alpha x) \sin(\beta y) \\
 V_{mn} \sin(\alpha x) \cos(\beta y) \\
 W_{mn} \sin(\alpha x) \sin(\beta y) \\
 X_{mn} \sin(\alpha x) \sin(\beta y) \\
 Z_{mn} \sin(\alpha x) \sin(\beta y)
\end{cases} (20)$$

where

$$\alpha = m\pi / a, \qquad \beta = n\pi / b \tag{21}$$

 $(U_{mn}, V_{mn}, W_{mn}, X_{mn}, Z_{mn})$ are the unknown maximum displacement coefficients. The transverse load q(x, y) is also expanded as

$$q(x,y) = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} Q_{mn} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}$$
 (22)

The coefficients Q_{mn} are given below for some typical loads

$$Q_{mn} = \frac{4}{ab} \int_{a}^{a} \int_{b}^{b} q(x, y) \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b} dxdy$$
 (23)

For uniformly distributed load

$$Q_{mn} = \begin{cases} \frac{16q_0}{mn\pi^2} & m, n = 1,3,5,...\\ 0 & m, n = 2,4,6,... \end{cases}$$
 (24)

For sinusoidal distributed load; $Q_{mn} = q_0$ in which q_0 is the intensity of the load.

Substituting Eqs. (21) and (22) into Eq. (18), the analytical solutions can be determined by

$$\begin{bmatrix}
a_{11} & a_{12} & a_{13} & a_{14} & a_{15} \\
a_{12} & a_{22} & a_{23} & a_{24} & a_{25} \\
a_{13} & a_{23} & a_{33} & a_{34} & a_{35} \\
a_{14} & a_{24} & a_{34} & a_{44} & a_{45} \\
a_{15} & a_{25} & a_{35} & a_{45} & a_{55}
\end{bmatrix}
\begin{bmatrix}
U_{mm} \\ V_{mm} \\ W_{mn} \\ X_{mm} \\ Z_{mm}
\end{bmatrix} = \begin{bmatrix}
0 \\
0 \\
Q_{mm} \\
0 \\
0$$
(25)

where

Table 1 Material properties used in the functionally graded sandwich plates

Properties -	(Al/2	ZrO ₂)	$(T_i$ -6Al-4V/ZrO ₂)		
	Zirconia	Aluminum	Zirconia	Titanium	
E (GPa)	151	70	117	66.2	
v	0.3	0.3	1/3	1/3	

$$a_{11} = -\left(A_{11}\alpha^{2} + A_{66}\beta^{2}\right)$$

$$a_{12} = -\alpha\beta \left(A_{12} + A_{66}\right)$$

$$a_{13} = \alpha\left(B_{11}\alpha^{2} + B_{12}\beta^{2} + 2B_{66}\beta^{2}\right)$$

$$a_{14} = \alpha\left(k_{1}B_{11}^{s} + k_{2}B_{12}^{s} - \left(k_{1}A^{'} + k_{2}B^{'}\right)B_{66}^{s}\beta^{2}\right)$$

$$a_{15} = X_{13}\alpha$$

$$(26)$$

$$a_{22} = -\left(A_{66}\alpha^{2} + A_{22}\beta^{2}\right)$$

$$a_{23} = \beta\left(B_{22}\beta^{2} + B_{12}\alpha^{2} + 2B_{66}\alpha^{2}\right)$$

$$a_{24} = \beta\left(k_{2}B_{22}^{s} + k_{1}B_{12}^{s} - \left(k_{1}A^{'} + k_{2}B^{'}\right)B_{66}^{s}\alpha^{2}\right)$$

$$a_{25} = X_{23}\beta$$

$$a_{33} = -\left(D_{11}\alpha^{4} + 2(D_{12} + 2D_{66})\alpha^{2}\beta^{2} + D_{22}\beta^{4}\right)$$

$$-k_{w} - k_{s}^{1}\alpha^{2} - k_{s}^{2}\beta^{2}$$

$$a_{34} = -k_{1}\left(D_{11}^{s}\alpha^{2} + D_{12}^{s}\beta^{2}\right) + 2(k_{1}A^{'} + k_{2}B^{'})D_{66}^{s}\alpha^{2}\beta^{2}$$

$$-k_{2}\left(D_{22}^{s}\beta^{2} + D_{12}^{s}\alpha^{2}\right)$$

$$a_{35} = -\left(Y_{13}\alpha^{2} + Y_{23}\beta^{2}\right)$$

$$a_{44} = -k_{1}\left(H_{11}^{s}k_{1} + H_{12}^{s}k_{2}\right) - (k_{1}A^{'} + k_{2}B^{'})^{2}H_{66}^{s}\alpha^{2}\beta^{2}$$

$$-k_{2}\left(H_{12}^{s}k_{1} + H_{22}^{s}k_{2}\right) - (k_{1}A^{'} + k_{2}B^{'})^{2}A_{55}^{s}\alpha^{2}$$

Table 2 Non dimensional center displacement \overline{w} and non-dimensional axial stress $\overline{\sigma_x}$ of (Al/ZrO₂) FG sandwich square plate under sinusoidally distributed load (a/h=10)

	Theory	C	\overline{w}				$\overline{\sigma_{\chi}}$			
		ε_z	2-1-2	1-1-1	2-2-1	1-2-1	2-1-2	1-1-1	2-2-1	1-2-1
0	Neves et al. 2012c	=0	0.19610	0.19610	0.19610	0.19610	1.99470	1.99470	1.99460	1.99460
	Neves et al. 2012c	$\neq 0$	0.19490	0.19490	0.19490	0.19490	2.00660	2.00660	2.00650	2.00640
	Bessaim et al. 2013	$\neq 0$	0.19486	0.19486	0.19486	0.19486	1.99524	1.99524	1.99524	1.99524
	Akavci 2016	=0	0.19605	0.19605	0.19605	0.19605	1.99516	1.99516	1.99516	1.99516
	Akavci 2016	$\neq 0$	0.19466	0.19466	0.19466	0.19466	2.0730	2.0730	2.0730	2.0730
	Present	=0	0.19606	0.19606	0.19606	0.19606	1.99332	1.99332	1.99332	1.99332
	Present	$\neq 0$	0.19487	0.19487	0.19487	0.19487	1.99525	1.99525	1.99525	1.99525
	Neves et al. 2012c	=0	0.30900	0.29490	0.28380	0.27400	1.47420	1.40670	1.30260	1.30640
	Neves et al. 2012c	$\neq 0$	0.30700	0.29290	0.28200	0.27220	1.48130	1.41370	1.30920	1.31330
	Bessaim et al. 2013	$\neq 0$	0.30430	0.29007	0.27874	0.26915	1.46131	1.39243	1.28274	1.29030
1	Akavci 2016	=0	0.30627	0.29196	0.28083	0.27093	1.46322	1.39432	1.28879	1.29201
	Akavci 2016	$\neq 0$	0.30398	0.28977	0.27847	0.26891	1.52514	1.45397	1.34177	1.34783
	Present	=0	0.30635	0.29202	0.28085	0.27093	1.46214	1.39324	1.28769	1.29091
	Present	$\neq 0$	0.30431	0.29007	0.27875	0.26915	1.46132	1.39244	1.28272	1.29030
	Neves et al. 2012c	=0	0.35420	0.33510	0.31860	0.30530	1.69200	1.60170	1.44760	1.45880
	Neves et al. 2012c	$\neq 0$	0.35190	0.33290	0.31640	0.30320	1.69940	1.60880	1.45430	1.46590
	Bessaim et al. 2013	$\neq 0$	0.35001	0.33068	0.31356	0.30060	1.68472	1.59170	1.42887	1.44497
2	Akavci 2016	=0	0.35222	0.33282	0.31613	0.30261	1.68708	1.59420	1.43723	1.44736
	Akavci 2016	$\neq 0$	0.34957	0.33030	0.31319	0.30031	1.75757	1.66237	1.49644	1.51084
	Present	=0	0.35237	0.33294	0.31620	0.30262	1.68601	1.59310	1.43607	1.44622
	Present	$\neq 0$	0.35001	0.33068	0.31356	0.30060	1.68473	1.59171	1.42886	1.44497
	Neves et al. 2012c	=0	0.40510	0.38680	0.36370	0.35030	1.93160	1.84850	1.63270	1.67610
	Neves et al. 2012c	$\neq 0$	0.40260	0.38430	0.36120	0.34800	1.93970	1.85590	1.63950	1.68320
	Bessaim et al. 2013	$\neq 0$	0.40153	0.38303	0.35885	0.34591	1.93266	1.84705	1.61792	1.66754
10	Akavci 2016	=0	0.40390	0.38538	0.36204	0.34817	1.93451	1.84956	1.62871	1.67048
	Akavci 2016	$\neq 0$	0.40094	0.38248	0.35823	0.34549	2.01036	1.92481	1.69436	1.74262
	Present	=0	0.40426	0.38566	0.36221	0.34830	1.93354	1.84857	1.62753	1.66937
	Present	$\neq 0$	0.40153	0.38303	0.35885	0.34591	1.93266	1.84707	1.61792	1.66755

$$-(k_{2}B')^{2}A_{44}^{s}\beta^{2}$$

$$a_{45} = -k_{1}A'A_{55}^{s}\alpha^{2} - k_{2}B'A_{44}^{s}\beta^{2} + k_{1}Y_{13}^{s} + k_{2}Y_{23}^{s}$$

$$a_{55} = -(A_{55}^{s}\alpha^{2} + A_{44}^{s}\beta^{2} + Z_{33})$$
(26)

4. Numerical results and discussions

In this section, the accuracy of the presented quasi-3D hyperbolic plate theory for the bending results of simply supported FG sandwich plates resting on elastic foundations and under sinusoidal loads is demonstrated by comparing the analytical solution with the existing results of quasi-3D and 2D shear theories in literature. In addition, the influences of shear deformation and thickness stretching on the bending response of the FGM sandwich plates are investigated. Typical values for metal and ceramics used in the FG sandwich plate The FG sandwich plates are made of (ZrO₂/Al) and (ZrO₂/T_i-6Al-4V), whose material properties are listed in Table 1.

For convenience, the following dimensionless forms are used

$$\overline{z} = \frac{z}{h}, \quad \overline{w} = \frac{10hE_0}{a^2q_0} w \left(\frac{a}{2}, \frac{b}{2}, \overline{z}\right),
\overline{\sigma}_x = \frac{10h^2}{a^2q_0} \sigma_x \left(\frac{a}{2}, \frac{b}{2}, \overline{z}\right), \quad \overline{\tau}_{xz} = \frac{h}{aq_0} \tau_{xz} \left(0, \frac{b}{2}, \overline{z}\right),$$

$$\widehat{w} = \frac{10^2 D_c}{a^4 q_0} w \left(\frac{a}{2}, \frac{b}{2}, \frac{z}{z} \right), \quad \widehat{\sigma}_x = \frac{1}{10^2 q_0} \sigma_x \left(\frac{a}{2}, \frac{b}{2}, \frac{z}{z} \right),$$

$$\widehat{\tau}_{xz} = \frac{1}{10q_0} \tau_{xz} \left(0, \frac{b}{2}, \frac{z}{z} \right), \quad K_w = \frac{k_w a^4}{D_c}, \quad K_s = \frac{k_s a^2}{D_c},$$

$$D_c = \frac{E_c h^3}{12(1 - v^2)}, \quad E_0 = 1 \text{ GP a}$$

The first example aims to verify the accuracy of the present theory in predicting the bending responses of FG sandwich plates. Table 2 contain the non-dimensional transverse displacement \overline{w} of the mid-plane and non dimensional axial stresses $\overline{\sigma_x}$ of the FG sandwich plate made of (ZrO₂/Al) under sinusoidal loads for different skincore-skin ratios and material parameter k. The obtained predictions are compared with the 2D and Quasi-3D solution of Neves et al. (2012c), Akavci (2016), and the quasi- 3D solutions of Bessaim et al. (2013). It should be noted that the quasi-3D solutions of Neves et al. (2012c), Akavci (2016) and Bessaim et al. (2013) are derived based on a hyperbolic variation of both in-plane and transverse displacements. The results of 2D solutions the present work, Neves et al. (2012c) and Akavci (2016) are provided to show the importance of including the thickness stretching effect. It can be seen that the dimensionless displacement and stresses predicted by the present new quasi-3D hyperbolic theory are in excellent agreement quasi-3D solutions, particularly with those reported by Bessaim et al.

Table 3 Nondimensional transverse shear stress $\bar{\tau}_{xz}$ for square (Al/ZrO₂) FG sandwich plate (a/h = 10)

-	Theory	$arepsilon_z$	$ar{ au}_{xz}$				
p			2-1-2	1-1-1	2-2-1	1-2-1	
0	Neves et al. 2012c	=0	0.2538	0.2459	0.2407	0.2358	
	Neves et al. 2012c	$\neq 0$	0.2538	0.2461	0.2411	0.2363	
	Bessaim et al. 2013	$\neq 0$	0.23794	0.23794	0.23794	0.23794	
	Present	=0	0.23217	0.23217	0.23217	0.23217	
	Present	$\neq 0$	0.23794	0.23794	0.23794	0.23794	
	Neves et al. 2012c	=0	0.2744	0.2640	0.2590	0.2489	
	Neves et al. 2012c	$\neq 0$	0.2745	0.2643	0.2594	0.2496	
1	Bessaim et al. 2013	$\neq 0$	0.27050	0.26060	0.25890	0.25196	
	Present	=0	0.26535	0.25533	0.25337	0.24635	
	Present	$\neq 0$	0.27050	0.26060	0.25890	0.25196	
	Neves et al. 2012c	= 0	0.2758	0.2664	0.2632	0.2515	
	Neves et al. 2012c	$\neq 0$	0.2760	0.2668	0.2636	0.2523	
2	Bessaim et al. 2013	$\neq 0$	0.28792	0.27138	0.26885	0.25776	
	Present	=0	0.28335	0.26661	0.26359	0.25241	
	Present	$\neq 0$	0.28792	0.27138	0.26885	0.25776	
	Neves et al. 2012c	= 0	0.2669	0.2635	0.2690	0.2559	
	Neves et al. 2012c	$\neq 0$	0.2671	0.2639	0.2692	0.2568	
10	Bessaim et al. 2013	$\neq 0$	0.33210	0.29534	0.29036	0.26850	
	Present	=0	0.32826	0.29167	0.28561	0.26392	
	Present	$\neq 0$	0.33210	0.29534	0.29036	0.26850	

Table 4 Non-dimensional center displacement \hat{w} of FG (T_i -6Al-4V/ZrO₂) sandwich plate on elastic foundation under sinusoidally distributed load (a/h = 10, b = 2a)

Scheme	r	Theory	C		(K_w, K_s)			
Scheme	p		$\boldsymbol{arepsilon}_{z}$	0, 0	100, 0	0, 100	100, 100	
1-0-1		Taibi <i>et al</i> . 2015	=0	0,681308	0,405225	0,0836524	0,077194	
	0	Akavci 2016	$\neq 0$	0,677195	0,404967	0.0728693	0,067958	
	U	Present	=0	0.681329	0.405232	0.0724390	0.067546	
		Present	$\neq 0$	0.678245	0.405328	0.072876	0.067960	
		Taibi <i>et al</i> . 2015	=0	0,886739	0,469985	0,0861015	0,079275	
	0.5	Akavci 2016	$\neq 0$	0.881167	0,470028	0.0747292	0,0695684	
1-0-1	0.5	Present	=0	0.886715	0.469978	0.0742679	0.0691335	
		Present	$\neq 0$	0.882541	0.470413	0.0747383	0.0695762	
		Taibi <i>et al</i> . 2015	=0	1,109938	0,526052	0,087816	0.080727	
	2	Akavci 2016	$\neq 0$	1,10267	0.526445	0.0760263	0,0706912	
	2	Present	=0	1.10979	0.526019	0.0755397	0.0702342	
		Present	$\neq 0$	1.104593	0.526872	0.0760337	0.0706977	
		Taibi <i>et al</i> . 2015	=0	0,868596	0.464839	0.085927	0.079128	
	0.5	Akavci 2016	$\neq 0$	0,86314	0.464849	0.0745969	0,0694537	
	0.3	Present	=0	0.868577	0.464833	0.0741382	0.0690211	
212		Present	$\neq 0$	0.864416	0.465262	0.0746036	0.0694619	
3-1-3		Taibi <i>et al</i> . 2015	=0	1,08997	0,519461	0,0876306	0.0805702	
	2	Akavci 2016	$\neq 0$	1.07386	0,519785	0.0758855	0,0705695	
		Present	=0	1.08085	0.519427	0.075402	0.0701153	
		Present	$\neq 0$	1.075741	0.520220	0.0758944	0.0705772	
		Taibi <i>et al</i> . 2015	=0	0,8604107	0,462484	0,0858464	0.079059	
	0.5	Akavci 2016	$\neq 0$	0.855014	0,462481	0,0745356	0.0694005	
2.1.2		Present	=0	0.8603967	0.462480	0.0740781	0.0689690	
		Present	$\neq 0$	0.856336	0.462864	0.074545	0.0694089	
2-1-2		Taibi <i>et al</i> . 2015	=0	1066384	0,516062	0,0875334	0.080488	
	2	Akavci 2016	$\neq 0$	1,05934	0,516358	0,0758117	0,705056	
	2	Present	=0	1.066248	0.516031	0.0753303	0.070053	
		Present	$\neq 0$	1.061188	0.516793	0.0758212	0.0705140	
1-1-1 -		Taibi <i>et al</i> . 2015	=0	0,838977	0,456219	00856283	0,0788745	
	0.5	Akavci 2016	$\neq 0$	0,833746	0,456185	0.0743699	0.0692568	
		Present	=0	0.838979	0.456220	0.0739157	0.0688281	
		Present	$\neq 0$	0.8350130	0.4565615	0.0743797	0.0692654	
		Taibi <i>et al</i> . 2015	=0	1,024387	0,506023	0,0872398	0,0872398	
	2	Akavci 2016	$\neq 0$	1.01766	0.506246	0,0755889	0,0703128	
	2	Present	=0	1.024298	0.506002	0.0751129	0.0698651	
		Present	$\neq 0$	1.019386	0.506675	0.0755999	0.0703225	

2013. Since the present quasi-3D theory and other quasi-3D theories include the thickness stretching effect, their solutions are very close to each other. However, the 2D solutions, which omits this effect, gives inaccurate prediction and slightly overestimates the deflection. Also, it can be observed from the Table 2 that the quasi-3D solutions, obtained lower transverse displacement and higher axial stress than the 2D solutions which eliminate the

stretching effect and increasing value of the power-law exponent p increases the center displacement in all sequences. The difference between shear deformation theories is less significant when $\varepsilon_z = 0$ especially for fully ceramic plates (p = 0).

Table 4 presents values of transverse shear stress $\bar{\tau}_{xz}$ for p=0, 1, 2, and 10 and different types of sandwich plates. The obtained results are compared with the different

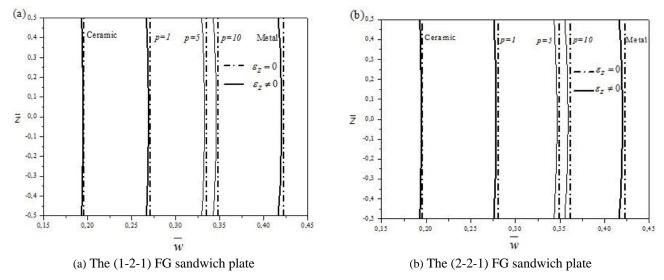


Fig. 3 The transverse displacement, \overline{w} , through the thickness of symmetric and unsymmetric sandwich square plates (a/h = 10)

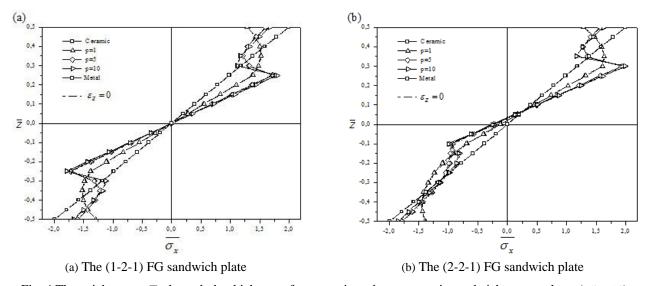


Fig. 4 The axial stress, \overline{w} , through the thickness of symmetric and unsymmetric sandwich square plates (a/h = 10)

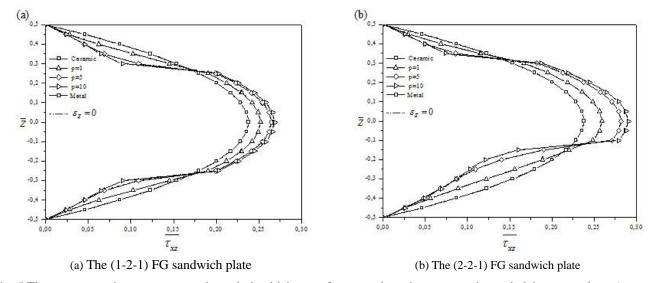


Fig. 5 The transverse shear stress, $\bar{\tau}_{xz}$, through the thickness of symmetric and unsymmetric sandwich square plates (a/h = 10)

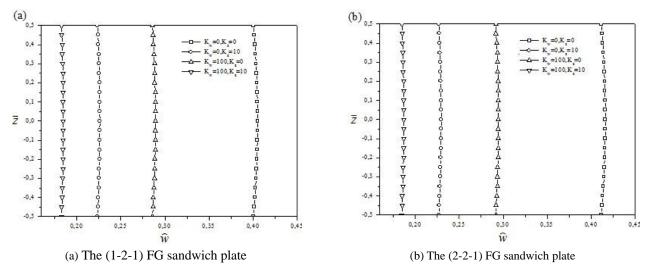


Fig. 6 Effect of Winkler and Pasternak modulus parameter on the dimensionless center deflection \hat{w} through the thickness of a square FG sandwich plate (p = 5, a/h = 10)

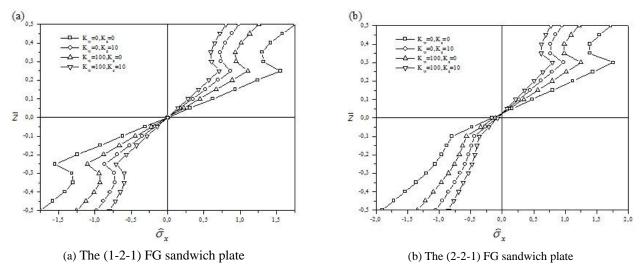


Fig. 7 Effect of Winkler and Pasternak modulus parameter on the dimensionless axial stress $\hat{\sigma}_x$ through the thickness of a square FG sandwich plate (p = 5, a/h = 10)

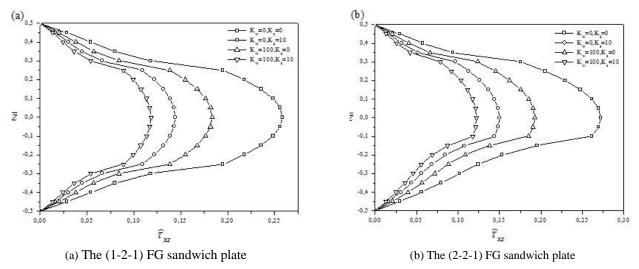


Fig. 8 Effect of Winkler and Pasternak modulus parameter on the dimensionless transverse shear stress $\hat{\tau}_{xz}$ through the thickness of a square FG sandwich plate (p = 5, a/h = 10)

two (Neves *et al.* 2012c) and quasi-three dimensional theories (Neves *et al.* 2012c, Bessaim *et al.* 2013). There is a little difference between the results and this is due to the different approaches used to predict the response of the FG sandwich plate. But, in general, a good agreement between the results is found. The transverse shear stress $\bar{\tau}_{xz}$ increases as p increases.

Fig. 3 show the influence of the volume fraction index p on the variation of the out-of-plane displacement (\overline{w}) through the thickness direction for both symmetric and unsymmetric square FG sandwich plates with side-to-thickness ratio a/h=10. The results are plotted by using both the present 2D and quasi-3D shear deformation theories. It can be seen that the transverse displacement (\overline{w}) of metal plates is larger than the corresponding one of ceramic plates and in general, the transverse displacement increases as the volume fraction index p increases.

Fig. 4 contain plot of the axial stress $\overline{\sigma_x}$ for various values of volume fraction index p through-the-thickness of rectangular FGM sandwich plate for both symmetric and unsymmetric square FG sandwich plates with side-to-thickness ratio a/h = 10. The maximum compressive stresses occur at a point on the top surface and the maximum tensile stresses occur, of course, at a point on the bottom surface of the FGM sandwich plate.

The homogeneous ceramic plate or metal plate yields the maximum compressive stresses at the bottom surface and the minimum tensile stresses at the top surface of the sandwich plate.

In Fig. 5 we have plotted the through-the-thickness distributions of the transverse shear stress the maximum value occurs at a point on the mid-plane of the plate for symmetric or homogeneous plates. It can be observed that the transverse shear stresses for non-symmetric FG sandwich plates are not parabolic and increasing the volume fraction index p leads to increase of the transverse shear stress in the skin of the plate which can increase the resistance of sandwich plates to face sheet debonding.

The second example to prove the validity of present Quasi-3D hyperbolic plate theory for bending response of a simply supported (T_i -6Al-4V/ZrO₂) FG sandwich plate resting on elastic foundation, the obtained numerical results presented in Table 4 and compared with the 2D solutions of Taibi *et al.* (2015) and the quasi-3D solutions of Akavci (2016). Additional results are plotted in Figs. 6–8 using the present Quasi-3D hyperbolic plate theory with ($\varepsilon_z \neq 0$).

Table 4 contain dimensionless center deflection \widehat{w} for an FG sandwich plate subjected to mechanical loads without elastic foundation or resting on one- or two-parameter elastic foundations for different values of volume fraction index p and several kinds of FG sandwich plates. It can be seen that the present results are in agreement with the published results for a simply supported FG sandwich plate on elastic foundation. As the volume fraction index p increases for FG plates, the deflection \widehat{w} will increase. Also, the inclusion of the Winkler foundation parameter gives results more than those with the inclusion of Pasternak foundation parameters. It can be shown that the deflections are decreasing with the existence of the elastic foundations. Since the quasi-3D model of the present

formulation includes the thickness stretching effect, the non-dimensional center deflection \widehat{w} are slightly decreased with respect to other center deflection documented in Table 4 obtained by 2D solutions. Thus, the inclusion of thickness stretching effect makes the FG sandwich plates stiffer.

The effect of foundation stiffness and side-to-thickness ratio on the dimensionless deflection \widehat{w} for both symmetric and unsymmetric FG sandwich square plate (k=5, a/h=10) is shown in Fig. 6. It is seen from Fig. 6 that as the elastic foundation increase the center dimensionless deflection \widehat{w} of the FG sandwich plates decreases. Figs. 7 and 8 plot the influences of foundation stiffness on the dimensionless axial stress $\widehat{\sigma}_x$ and the transverse shear stress $\widehat{\tau}_{xz}$ through-the-thickness of the symmetric and unsymmetric FG sandwich square plate (k=5, a/h=10). It is seen from the figures that the axial normal and transverse shear stresses decrease gradually with the increasing value of foundation stiffness.

5. Conclusions

A novel Quasi-3D hyperbolic shear deformation plate theory is developed for bending response of FG sandwich plates resting on elastic foundation. By considering further simplifying suppositions to the existing Quasi-3D theory, with incorporation of an undetermined integral term, the present theory has only five unknowns, which is even less than the other shear and normal deformation theories, and hence, make this model simple and efficient to employ. Equations of motion are obtained by utilizing the Hamilton's principles and then are solved using Navier's procedure. The accuracy of the present work is ascertained by comparing it with existing solutions and excellent agreement was observed. Results show that the inclusion of thickness stretching effect ($\varepsilon_z \neq 0$) makes a nanoplates stiffer, and hence, leads to decrease of the transverse displacement. In conclusion, it can be said that the present theory is not only accurate but also simple in predicting displacements and stresses of FG sandwich plates without elastic foundation or resting on one- or two-parameter elastic foundations. The formulation lend sit self particularly well to study several problems related to the hygro-thermo-mechanical deformation of laminated and FG structures as is studied by Tounsi and his co-workers (Bouderba et al. 2016, Beldjelili et al. 2016, Bousahla et al. 2016, Chikh et al. 2017), also by including the sizedependent effect for analysis of mechanical behaviour of micro/structure (Adda Bedia et al. 2015, Belkorissat et al. 2015, Larbi Chaht et al. 2015, Kolahchi and Bidgoli 2016, Arani and Kolahchi 2016, Bouafia et al. 2017), which will be considered in the near future.

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